GENERAL NOTES

THE STRUCTURAL DRAWINGS DEPICT THE STRUCTURE IN ITS FINAL CONSTRUCTED CONFIGURATION. NEITHER CONSTRUCTION MEANS AND METHODS NOR CONSTRUCTION SAFETY ARE PART OF THE STRUCTURAL ENGINEER'S EXPERTISE OR SCOPE OF WORK. THE GENERAL CONTRACTOR AND THEIR SUBCONTRACTORS ARE FULLY RESPONSIBLE FOR THE MEANS AND METHODS USED TO CONSTRUCT THE STRUCTURE AND FOR FULL COMPLIANCE WITH ALL JOB SAFETY RELATED REGULATIONS AND CONDITIONS AT THE SITE. LIMITED SITE VISITS, IF ANY, BY THE STRUCTURAL ENGINEER ARE SOLELY TO OBSERVE COMPLETED PARTS OF THE STRUCTURE. THE STRUCTURAL ENGINEER IS NEITHER QUALIFIED TO OBSERVE NOR COMMENT ON CONSTRUCTION MEANS AND METHODS AND JOB SITE SAFETY.

CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO FABRICATION/CONSTRUCTION. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO FABRICATION/CONSTRUCTION. CONTRACTOR IS RESPONSIBLE FOR ALL WORK FITTING AS INTENDED BY THE DRAWINGS AND SPECIFICATIONS.

PRINCIPAL OPENINGS ARE SHOWN ON THE DRAWINGS. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENINGS, SLEEVES, CURBS, INSERTS, DEPRESSIONS, ETC. NOT SHOWN.

ALL DETAILS ARE TYPICAL UNLESS NOTED OTHERWISE. DETAILS SHALL APPLY TO SIMILAR AND LIKE CONDITIONS.

COLD-FORMED WALL STUDS AND CONNECTIONS SHALL BE DESIGNED AND SEALED BY A LICENSED ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. THE CALCULATIONS AND DETAIL DRAWINGS SHALL BE SUBMITTED FOR APPROVAL SHOWING ALL CALCULATIONS, INCLUDING DESIGN LOADS, MEMBER SIZES AND CONNECTIONS.

SHOP DRAWINGS SHALL BE NEW DRAWINGS PRODUCED BY THE CONTRACTOR, ILLEGIBLE REPRODUCTIONS OF THE DESIGN DRAWINGS WILL BE REJECTED. ELECTRONIC FILES MAY BE REQUESTED FROM THE ENGINEER OF RECORD FOR THE PURPOSE OF PREPARING SHOP DRAWINGS. THE CONTRACTOR WILL BE REQUIRED TO SIGN AN INDEMNITY STATEMENT PRIOR TO TRANSMISSION OF THE ELECTRONIC FILES TO THE CONTRACTOR. THE USE OF REPRODUCTIONS OF THESE CONTRACT DRAWINGS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREIN AS CORRECT, AND OBLIGATES THEMSELF TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS OR OMISSIONS THAT MAY OCCUR HEREIN.

MINIMUM SHOP DRAWING SUBMITTAL REQUIREMENTS INCLUDE:

CONCRETE MIX DESIGNS FOR EACH CLASS OF CONCRETE WITH TEST DATA
CONCRETE ACCESSORIES (VAPOR BARRIER, REINFORCING SUPPORT CHAIRS, ETC.)
CONCRETE REINFORCING SHOP DRAWINGS
STRUCTURAL STEEL SHOP DRAWINGS (SEALED BY A LICENSED PROFESSIONAL ENGINEER)
STRUCTURAL STEEL CONNECTIONS AND CALCULATIONS (SEALED BY A LICENSED PROFESSIONAL ENGINEER)
STEEL DECK SHOP DRAWINGS
COLD-FORMED METAL FRAMING SHOP DRAWINGS AND CALCULATIONS (SEALED BY A LICENSED PROFESSIONAL ENGINEER)
FRP (FIBER REINFORCED POLYMER) STRENGTHENING SHOP DRAWINGS AND CALCULATIONS (SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER). SEE FRP NOTES, THIS SHEET
SPECIAL SEISMIC CERTIFICATION FOR MECHANICAL EQUIPMENT
SPECIAL SEISMIC CERTIFICATION FOR ELECTRICAL EQUIPMENT

THE CONTRACTOR IS TO REVIEW EACH SUBMITTAL PRIOR TO FORWARDING TO THE ARCHITECT AND STRUCTURAL ENGINEER. THE CONTRACTOR IS TO STAMP EACH SUBMITTAL VERIFYING THAT THE FOLLOWING ITEMS ARE ADDRESSED:

THE SHOP DRAWING IS REQUESTED.
THE SHOP DRAWING IS BASED ON THE LATEST DESIGN.
THE ARCHITECT'S AND STRUCTURAL ENGINEER'S COMMENTS FROM ANY PREVIOUS SUBMITTALS ARE ADDRESSED.
THE WORK IS COORDINATED AMONGST ALL CONSTRUCTION TRADES.
REVISIONS FROM PREVIOUS SUBMITTALS ARE CLEARLY MARKED BY CLOUDS.
SUBMITTAL IS COMPLETE.
SUBMITTAL DOES NOT INCLUDE SUBSTITUTION REQUEST(S).
SUBMITTAL SHALL INCLUDE A STAMP INDICATING PROJECT NAME AND LOCATION, SUBMITTAL NUMBER, AND SPECIFICATION SECTION NUMBER.

THE STRUCTURAL ENGINEER SHALL RETURN, WITHOUT COMMENT, SUBMITTALS WHICH THE CONTRACTOR HAS NOT STAMPED OR WHICH DO NOT MEET THE ABOVE REQUIREMENTS. THE STRUCTURAL ENGINEER' S REVIEW OF SUBMITTALS SHALL BE FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT. NO WORK SHALL BE STARTED WITHOUT SUCH REVIEW.

CRANES, CONCRETE TRUCKS AND ALL OTHER HEAVILY LOADED VEHICLES ARE NOT TO BE DRIVEN ACROSS GRADE BEAMS OR BUILDING SLABS.

ERECTION OF STRUCTURAL STEEL MAY NOT BEGIN UNTIL CONCRETE FOUNDATION HAS CURED FOR A MINIMUM OF THREE (3) DAYS. STRUCTURAL STEEL OR OTHER HEAVY LOADS SHALL NOT BE STOCKPILED ON ANY SLAB UNTIL IT HAS CURED FOR A MINIMUM OF THREE (3) DAYS.

THIS PROJECT CONSISTS OF A MODIFICATION TO AN EXISTING BUILDING. "AS-BUILT" DRAWINGS FOR THE EXISTING BUILDING WERE NOT AVAILABLE DURING DESIGN. EXISTING DIMENSIONS, FRAMING LAYOUT AND MEMBER SIZES WERE DETERMINED BY VISUAL OBSERVATION ONLY WITHOUT THE REMOVAL OF FINISHES AND THEREFORE THE ACCURACY OF THIS INFORMATION IS NOT GUARANTEED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL EXISTING DIMENSIONS SHOWN ON THESE DRAWINGS AND TO VERIFY THE LOCATION OF ALL FRAMING MEMBERS AND OTHER OBSTRUCTIONS WHICH WILL AFFECT THE WORK. AS PART OF THIS WORK THE CONTRACTOR SHALL PREPARE AN ACCURATE FIELD SURVEY OF THE LOCATION OF ALL STRUCTURAL MEMBERS IN THE WORK AREA PRIOR TO BEGINNING SHOP DRAWINGS AND CONSTRUCTION. THIS SURVEY SHALL BE SUBMITTED TO THE ARCHITECT WITH VARIANCES NOTED. CLAIMS FOR ADDITIONAL TIME OR EXTRA COST DUE TO OBSTRUCTIONS AND VARIANCES IN THE LOCATION OF THE STRUCTURAL MEMBERS WILL NOT BE HONORED AFTER WORK HAS BEGUN ON THE PROJECT.

BUILDING CODE: INTERNATIONAL BUILDING CODE, 2021 EDITION

DEAD LOAD:
ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD CARRYING CAPACITY OF THE STRUCTURE.

LIVE LOAD:

ROOF:	20 PSF
STAIRS, EXITWAYS:	100 PSF
PARTITION:	20 PSF
CORRIDORS:	100 PSF (1ST FLOOR)/ 80 PSF (ABOVE 1ST FLOOR)
LOBBIES:	100 PSF
ADMIN/OFFICE:	80 PSF
LIGHT STORAGE:	125 PSF
MECHANICAL:	150 PSF
FILE/COMPUTER ROOMS:	125 PSF
HANDRAILS:	50 PLF / 200 POUNDS IN ANY DIRECTION

LIVE LOAD REDUCTIONS HAVE BEEN APPLIED IN ACCORDANCE WITH THE BUILDING CODE, UNLESS NOTED OTHERWISE.

ARCHITECTURAL BARRIER ACT:

GRAB BAR:	250 LBS ANY DIRECTION
TUB/SHOWER SEAT:	250 LBS ANY DIRECTION
FASTENERS:	250 LBS ANY DIRECTION

WIND LOAD:
INTERNATIONAL BUILDING CODE, 2021 EDITION

VELOCITY (ULT): 123 MPH THREE SECOND GUST
EXPOSURE: C (NORTH ELEVATION)
RISK CATEGORY: IV
INTERNAL PRESSURE COEFFICIENT, G_{cpi}: +/- 0.18

MAXIMUM GROSS UPLIFT INTERIOR ZONE: ±30 PSF
MAXIMUM GROSS UPLIFT EXTERIOR ZONE: ±20 PSF
18'-0" FROM EACH CORNER)

SEISMIC LOAD:

RISK CATEGORY:	IV
SEISMIC IMPORTANCE FACTOR (I):	1.5
MAPPED SPECTRAL RESP ACCEL (S_s/S_1):	0.091g/0.055g
SITE CLASS:	D
SPECTRAL RESPONSE COEFF (S_{ds}/S_{d1}):	0.097g/0.089g
SEISMIC DESIGN CATEGORY:	C
SEISMIC FORCE RESISTING SYSTEM:	R=3.0, STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMNS
DESIGN BASE SHEAR:	33.1 KIP
SEISMIC RESPONSE COEFF (C_s):	0.049
ANALYSIS PROCEDURE USED:	EQUIVALENT LATERAL FORCE

SNOW LOAD:

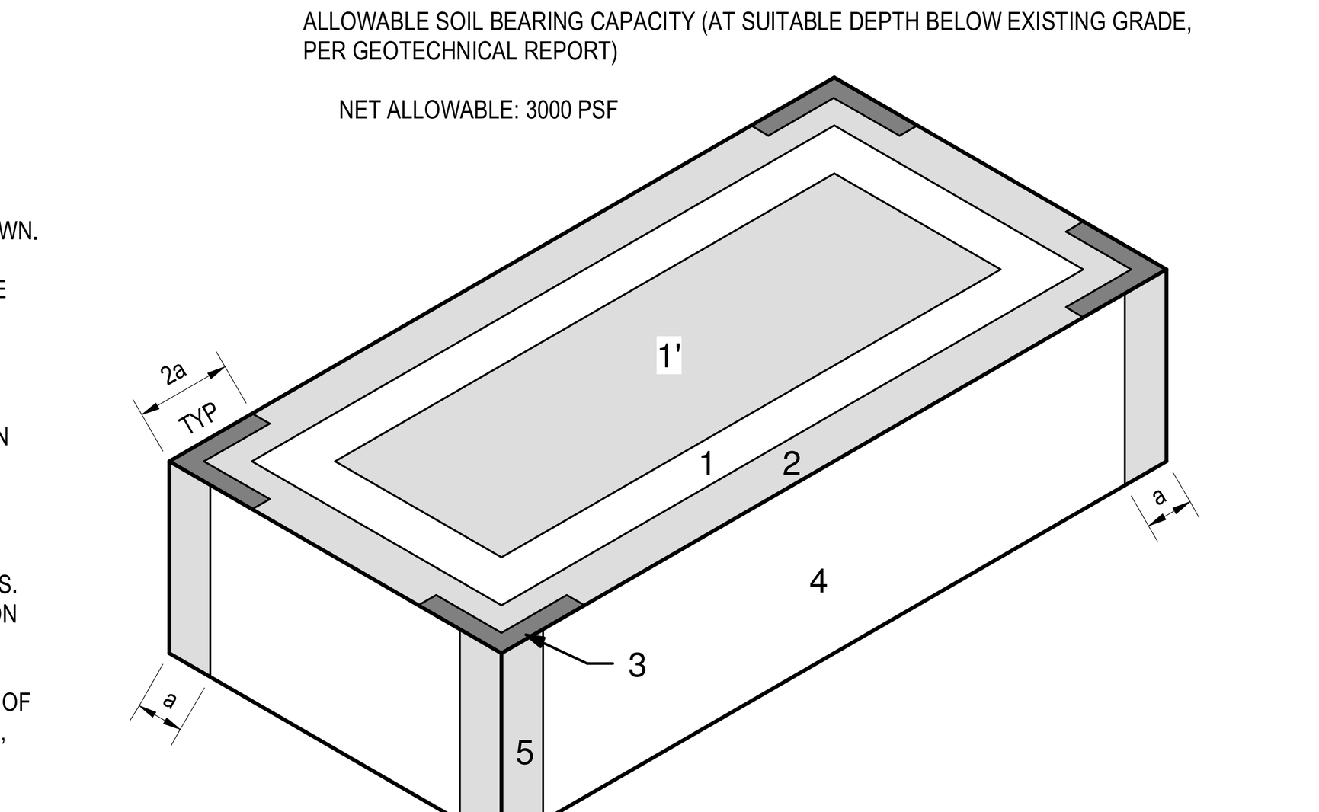
GROUND SNOW LOAD (P_g):	15 PSF
FLAT ROOF SNOW LOAD (P_f):	15.1 PSF
SNOW EXPOSURE FACTOR (C_e):	1.2
SNOW LOAD IMPORTANCE FACTOR (I):	1.2
THERMAL FACTOR (C_t):	1.0

BLAST LOAD:
ROOF/PENTHOUSE FRAMING NOT INCLUDED IN LATERAL SYSTEM
DESIGN LEVEL VEHICLE THREAT: W1
STANDOFF DISTANCE: 50 FT

ROOF/PENTHOUSE LATERAL FRAME RESPONSE SYSTEM
DESIGN LEVEL VEHICLE THREAT: W2
STANDOFF DISTANCE: 50 FT

ALLOWABLE SOIL BEARING CAPACITY (AT SUITABLE DEPTH BELOW EXISTING GRADE, PER GEOTECHNICAL REPORT)

NET ALLOWABLE: 3000 PSF



2a = 20'-6"
a = 10'-3"

ROOF DESIGN GROSS PRESSURES, WIND ONLY (PSF)			
ZONE	EFFECTIVE WIND AREA ft²		
	20 ft²	50 ft²	100 ft²
1'	+/- 35	+/- 35	+/- 35
1	+/- 55	+/- 55	+/- 50
2	+/- 80	+/- 70	+/- 65
3	+/- 105	+/- 85	+/- 75

SITE DRAINAGE

1. DURING CONSTRUCTION, GRADE THE SITE TO PROVIDE POSITIVE DRAINAGE AWAY FROM ALL BUILDINGS AND SLABS. WATER SHALL NOT BE ALLOWED TO POND ADJACENT TO THE BUILDING FOUNDATIONS OR SLABS.
2. DOWNSPOUTS FROM ROOF DRAINS AND GUTTERS SHALL BE COLLECTED AND PIPED AWAY FROM THE BUILDING. WHEN WATER IS NOT PIPED WAY FROM THE BUILDING, DOWNSPOUTS SHALL DUMP ONTO A CAST IN PLACE 4" THICK X 3'-0" WIDE CONCRETE SWALE REINFORCED WITH #4 AT 12" ON CENTER EACH WAY AND EXTENDING 10'-0" OUT FROM THE BUILDING.
3. FINAL SITE GRADING SHALL BE IN COMPLIANCE WITH THE INTERNATIONAL BUILDING CODE SECTION 1804.4. PERVIOUS SURFACES ADJACENT TO BUILDINGS SHALL SLOPE AWAY FROM BUILDING AT 5% MINIMUM FOR AT LEAST 10 FEET. IMPERVIOUS SURFACES ADJACENT TO BUILDINGS SHALL SLOPE AWAY FROM BUILDING AT 2% MINIMUM FOR AT LEAST 5 FEET.
4. TREES AND VEGETATION SHALL NOT BE ALLOWED WITHIN A DISTANCE EQUAL TO THREE QUARTERS THEIR ULTIMATE HEIGHT AWAY FROM THE BUILDING.
5. IRRIGATE VEGETATION AND SOILS ADJACENT TO THE BUILDING (NO MORE THAN 15 MINUTES THREE TIMES A WEEK) ON AN AS NEEDED BASIS TO MAINTAIN UNIFORM SOIL MOISTURE CONDITIONS AROUND THE PERIMETER OF THE BUILDING FOLLOWING CONSTRUCTION.

EARTHWORK

1. WHERE CONSTRUCTION DOCUMENT REQUIREMENTS DIFFER FROM THE PROJECT SPECIFIC GEOTECHNICAL REPORT, THE MORE STRINGENT OF THE TWO SHALL GOVERN.
2. SITE PREPARATION FOR THE BUILDING PAD SHALL CONSIST OF THE REMOVAL OF EXISTING PAVEMENT, VEGETATION, ORGANIC MATTER AND ANY ADDITIONAL MATERIAL AS NECESSARY TO PROVIDE THE REQUIRED AMOUNT OF FILL UNDER THE BUILDING.
3. DURING THE GEOTECHNICAL EXPLORATION, EXISTING FILL MATERIALS WERE ENCOUNTERED FROM 2 TO 6 FEET BELOW EXISTING GRADE. ADDITIONAL FILL SHOULD BE EXPECTED TO OCCUR ACROSS THE SITE, POSSIBLY AT GREATER DEPTHS. THE EXISTING FILL IS NOT SUITABLE FOR THE BUILDING ADDITION AND SHALL BE REMOVED. DUE TO THE VARIABILITY OF THE EXISTING FILL DEPTH, THE EXACT AMOUNT OF FILL TO BE REMOVED CANNOT BE PREDICTED.
4. THE EXISTING FILL WHICH IS TO BE REMOVED CAN BE REUSED AS NEW CONTROLLABLE FILL BELOW THE LOW VOLUME CHANGE (LVC) ZONE, PROVIDED IT IS PROPERLY MOISTURE CONDITIONED AND COMPACTED.
5. THE SUBGRADE SHALL BE PROOFROLLED WITH A HEAVY, RUBBER-TIRED VEHICLE (STATIC WEIGHT OF AT LEAST 20 TONS AND WITH TIRE PRESSURES OF AT LEAST 90 PSI). THE CONTRACTOR SHALL MAKE AT LEAST TWO COMPLETE PASSES OVER THE AREA WITH THE SECOND PASS PERPENDICULAR TO THE FIRST PASS. AREAS OF THE SUBGRADE THAT ARE OBSERVED TO BE SOFT OR WEAK SHALL BE OVEREXCAVATED AND REPLACED WITH PROPERLY COMPACTED SELECT FILL.
6. SUBGRADE SHALL THEN BE SCARIFIED AND MOISTURE CONDITIONED TO AN SIX (6) INCH DEPTH AND THEN RECOMPACTED TO BETWEEN 95 AND 100 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR DENSITY TEST (ASTM D698). THE MOISTURE CONTENT SHALL BE BETWEEN OPTIMUM AND +3 PERCENT OF THE OPTIMUM MOISTURE CONTENT. PROVIDE A MINIMUM OF FOUR (4) FIELD DENSITY TESTS ON THE SUBGRADE OR ONE (1) FOR EVERY 2,500 SQUARE FEET WHICHEVER IS GREATER.
7. SELECT FILL MATERIAL FOR THE BUILDING PAD SHALL MEET THE FOLLOWING REQUIREMENTS:

SOIL TYPE	USCS CLASSIFICATION	ACCEPTABLE PARAMETERS
LEAN CLAY	CL (LL < 45 & PI > 15	> 24 INCHES BELOW BUILDING ADDITION FINISHED SUBGRADE
LEAN TO FAT CLAY	CL/CH (46 ≤ LL < 50)	> 24 INCHES BELOW BUILDING ADDITION FINISHED SUBGRADE
FAT CLAY	CH (LL ≥ 50)	> 24 INCHES BELOW BUILDING ADDITION FINISHED SUBGRADE
WELL-GRADED GRANULAR AND SILTY GRAVEL	GM-GW GM	ALL LOCATIONS AND ELEVATIONS
LOW VOLUME CHANGE (LVC) MATERIAL	CL OR GM-GW, GM AND (LL < 40 & 5 ≤ PI < 15)	ALL LOCATIONS AND ELEVATIONS

8. SELECT FILL SHALL BE PLACED IN NINE (9) INCH LOOSE LIFTS WITH HEAVY, SELF-PROPELLED COMPACTION EQUIPMENT AND SIX (6) INCH LOOSE LIFTS WITH HAND-GUIDED EQUIPMENT AND COMPACTED TO BETWEEN 98 AND 100 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR DENSITY TEST (ASTM D698). THE MOISTURE CONTENT SHALL BE AS OUTLINED BELOW. PROVIDE A MINIMUM OF (4) FIELD DENSITY TESTS ON EACH LIFT OF SELECT FILL OR ONE (1) TEST PER DAY.

SOIL TYPE	ACCEPTABLE MOISTURE CONTENT
COHESIVE SOILS W/ PI ≥ 35	AT LEAST 3 PERCENTAGE POINTS ABOVE OPTIMUM MOISTURE CONTENT VALUE AS BY STANDARD PROCTOR TEST AT TIME OF PLACEMENT AND COMPACTION
COHESIVE SOILS W/ 24 ≤ PI < 35	AT LEAST 2 PERCENTAGE POINTS ABOVE OPTIMUM MOISTURE CONTENT VALUE AS BY STANDARD PROCTOR TEST AT TIME OF PLACEMENT AND COMPACTION
COHESIVE SOILS W/ 18 ≤ PI < 24	ABOVE OPTIMUM MOISTURE CONTENT VALUE AS BY STANDARD PROCTOR TEST AT TIME OF PLACEMENT AND COMPACTION
COHESIVE SOILS W/ PI < 18	AT MOST 2 PERCENTAGE POINTS BELOW OPTIMUM MOISTURE CONTENT VALUE AS BY STANDARD PROCTOR TEST AT TIME OF PLACEMENT AND COMPACTION
GRANULAR MATERIAL	WORKABLE MOISTURE LEVELS

9. SELECT FILL MATERIAL SHALL BE TESTED DURING PLACEMENT OF EACH LIFT FOR THE ATTERBERG LIMITS IN ACCORDANCE WITH ASTM D4318-98 METHOD B "STANDARD TEST METHOD FOR LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS" TO VERIFY THAT THE SELECT FILL MATERIAL IS IN ACCORDANCE WITH THE ORIGINALLY APPROVED SELECT FILL MATERIAL. PROVIDE A MINIMUM OF ONE (1) TEST PER DAY.
10. CONTRACTOR SHALL MAINTAIN A CLEAN EXCAVATION THAT IS FREE OF WATER 100 PERCENT OF THE TIME. CONTRACTOR SHALL PROVIDE PUMPS AS REQUIRED TO REMOVE ANY WATER AT ALL TIMES.
11. THE SITE SHALL BE GRADED TO PROVIDE POSITIVE DRAINAGE AWAY FROM THE BUILDING PAD DURING BUILDING PAD INSTALLATION AND WHEN THE BUILDING PAD AND BUILDING ARE COMPLETED.
12. PLUMBING AND UTILITY TRENCHES WITHIN THE BUILDING PAD SHALL HAVE PIPING BEDDED ON 6" MINIMUM OF CEMENT STABILIZED SAND WITH 4" MINIMUM ALL AROUND. BACKFILL IN UTILITY TRENCHES SHALL CONSIST OF COMPACTED SELECT FILL. PROVIDE A BENTONITE PLUG FOR THE FULL DEPTH AND WIDTH OF THE UTILITY TRENCH TO A MINIMUM OF 1'-0" ABOVE THE BOTTOM OF THE FOUNDATION AT THE EXTERIOR FACE OF BUILDING FOUNDATIONS WHERE UTILITY TRENCHES ENTER THE BUILDING.
13. PROVIDE A MINIMUM SIX (6) INCH CLAY CAP FOR A MINIMUM OF 5'-0" AROUND THE PERIMETER OF THE BUILDING. THE CAP SHALL EXTEND AS REQUIRED TO COVER THE LIMITS OF THE EXCAVATION AND SELECT FILL BUILDING PAD MATERIALS.

CONCRETE

1. ALL CONCRETE REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, EXCEPT WHERE NOTED. NO. 3 BARS SHALL CONFORM TO ASTM A615, GRADE 40. DEFORMED BAR ANCHORS SHALL CONFORM TO ASTM A496, GR 70.
2. ALL WELDED WIRE FABRIC SHALL BE SMOOTH WIRE FABRIC CONFORMING TO ASTM A1064, AND SHALL BE FURNISHED IN FLAT SHEETS ONLY. ROLLED WIRE FABRIC WILL BE REJECTED.
3. CONCRETE IN THE FOLLOWING AREAS SHALL HAVE SAND AND CRUSHED CARBONATE AGGREGATE CONFORMING TO ASTM C33, TYPE 1 PORTLAND CEMENT, FLYASH CONFORMING TO ASTM C618, CLASS 'C' UP TO 20 PERCENT REPLACEMENT BY VOLUME AND THE FOLLOWING DESIGNATED COMPRESSIVE STRENGTH (f'c) IN 28 DAYS:
   - FOOTINGS ---------------------- 4000 PSI (w/c = 0.50 MAX)
   - SLABS ON GRADE ----------- 4000 PSI (w/c = 0.50 MAX)
   - ALL OTHER CONCRETE ---- 4000 PSI (w/c = 0.50 MAX)
     CONCRETE SUPPLIER SHALL BE AWARE OF CEMENTS THAT CAN CAUSE ETTRINGITE FORMATION IN THE CEMENT PASTE AND BE PREPARED TO SHOW THAT THE CEMENTS USED WILL NOT CAUSE THIS PROBLEM.
4. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE AS FOLLOWS; SEE SEC. 20.5 ACI 318, LATEST EDITION FOR CONDITIONS NOT NOTED, PROVIDE CHAIR SUPPORTS (AZTEC CASTLE CHAIR, WHC SERIES 'B' OR EQUAL) TO ADEQUATELY SUPPORT BARS FOR PROPER CLEARANCE AS RECOMMENDED BY THE AMERICAN CONCRETE INSTITUTE AND THE CONCRETE REINFORCING STEEL INSTITUTE. SLAB ON GRADE REINFORCEMENT SHALL BE SUPPORTED AT 45-INCH MAXIMUM INTERVALS OR EVERY THIRD BAR.
   - FOOTINGS ---- 3 IN.
   - GRADE BEAMS ---- 3 IN. BOT., 2 IN. SIDE (3" EARTH FORMED), 2 IN. TOP
   - SLAB ON GRADE ---- 1 IN. TOP
5. DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI PUBLICATION 315, LATEST EDITION. ALL HOOKED BARS SHOWN IN DETAILS SHALL HAVE STANDARD HOOKS UNLESS NOTED OTHERWISE.
6. ALL MIXING, TRANSPORTING, PLACING AND CURING OF CONCRETE SHALL BE DONE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE, ACI 301, LATEST EDITION.
7. NO HORIZONTAL JOINTS WILL BE PERMITTED IN CONCRETE EXCEPT WHERE THEY NORMALLY OCCUR OR WHERE NOTED. VERTICAL JOINTS SHALL OCCUR AT CENTER SPANS OR AT LOCATIONS APPROVED BY THE STRUCTURAL ENGINEER.
8. REINFORCING BARS SHALL NOT BE WELDED WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.
9. CONTINUOUS BOTTOM REINFORCING BARS SHALL BE SPLICED AT SUPPORTS AND CONTINUOUS TOP REINFORCING BARS SHALL BE SPLICED AT MID-SPAN, UNO.
10. ALL CONTINUOUS REINFORCEMENT SHALL HAVE CLASS B LAP AT SPLICES. PROVIDE (1)-#6 X 6'-0" TOP AND BOTTOM (TWO 36" LEGS WITH 90 DEGREE BEND) AT EACH FACE OF GRADE BEAMS AT CORNERS AND INTERSECTIONS, AND AT 18" ON CENTER VERTICALLY AT WALLS.
11. CONDUITS ARE NOT ALLOWED IN SLABS, BEAMS, WALLS OR COLUMNS. ALL CONDUITS SHALL BE SUSPENDED FROM OR ATTACHED TO THE CONCRETE STRUCTURE.
12. ALL BASE PLATES AND ANCHOR RODS SHALL BE PROTECTED WITH 3" (MIN.) OF CONCRETE. ANCHOR RODS SHALL BE FABRICATED FROM FULL BODIED STEEL RODS CONFORMING TO ASTM F1554 GR.36, WASHERS CONFORMING TO ASTM F844 AND NUTS CONFORMING TO ASTM A194 OR A563 AND HAVING THE SAME DIAMETER AS THE BOLT DIAMETER AND USING CUT THREADS. ROLLED THREADS ARE NOT ACCEPTABLE. BOLTS SHALL BE SET USING RIGID TEMPLATES.
13. PROVIDE (1)-#5 X 4'-0" AT RE-ENTRANT CORNERS AND AROUND RECTANGULAR HOLES IN SLABS UNLESS NOTED OTHERWISE. PLACE BAR DIAGONAL TO CORNER WITH 1" CLEARANCE FROM TOP AND THE SIDE OF THE SLAB AT THE CORNER.
14. WHERE FRESH CONCRETE IS DEPOSITED AGAINST HARDENED CONCRETE (GREATER THAN 8 HOURS OLD), CLEAN EXISTING SURFACE OF LAITANCE AND FOREIGN MATERIAL AND DAMPEN THE EXISTING SURFACE. IF REQUIRED, ROUGHEN EXISTING CONCRETE TO ¼" AMPLITUDE.
15. SAW CUT JOINTS OR KEYED CONSTRUCTION JOINTS IN SLABS ON GRADE SHALL BE SPACED TO DIVIDE THE SLAB INTO PANELS NOT TO EXCEED 225 SQUARE FEET. THE LONGER DIMENSION OF EACH PANEL SHALL NOT EXCEED THE SHORTER DIMENSION BY MORE THAN 20%. JOINTS SHALL BE LOCATED AT COLUMN CENTERLINES WHERE POSSIBLE. CONTRACTOR SHALL SUBMIT JOINT LAYOUT TO ARCHITECT FOR APPROVAL.

FOUNDATIONS

1. PREPARED GRADE AREA UNDER ALL BUILDING SLABS AND GRADE BEAMS SHALL BE COVERED WITH A 15 MIL WATER VAPOR BARRIER MEETING THE REQUIREMENTS OF ASTM E 1745 (LATEST EDITION), CLASS A OR BETTER WITH MAXIMUM WATER PERMEANCE OF 0.1 PERMS WHEN TESTED IN ACCORDANCE WITH ASTM E96. THE WATER VAPOR BARRIER SHALL BE INSTALLED AND LAPPED AND TAPED IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM E 1643 (LATEST EDITION). PENETRATIONS SHALL BE SEALED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND REQUIREMENTS.
2. FOUNDATION DETAILING SHOWN ON THE DRAWINGS IS BASED ON A FOUNDATION DESIGN SPECIFIED IN THE SOIL REPORT COMPLETED BY TERRACON CONSULTANTS, INC., REPORT NO. 01215004, DATED MAY 21, 2021. THE RECOMMENDATIONS CONTAINED IN THE SOIL REPORT SHALL NOT SUPERCEDE THE REQUIREMENTS SHOWN ON THE DESIGN DRAWINGS OR IN THE SPECIFICATIONS WHEN THE REQUIREMENTS SHOWN IN THE DRAWINGS ARE GREATER THAN THOSE SHOWN IN THE GEOTECHNICAL REPORT. THE CONTRACTOR IS REQUIRED TO SECURE A COPY OF THE GEOTECHNICAL REPORT FROM THE OWNER AND HAVE A COPY ON THE JOB SITE AT ALL TIMES FOR THEIR USE AND REFERENCE.
3. FOUNDATION DETAILING SHOWN ON THE DRAWINGS IS BASED ON SELECT FILL MATERIAL (PER EARTHWORK GENERAL NOTE #7) BENEATH THE FLOOR SLAB.
4. ALL BACKFILL FOR BURIED PIPES AND CONDUIT WITHIN THE BUILDING PAD AND EXTENDING OUT MINIMUM 5'-0" BEYOND THE BUILDING SHALL BE BACKFILLED WITH SELECT FILL BACKFILL. DO NOT USE SAND BACKFILL. A BENTONITE PLUG SHALL BE PROVIDED FOR THE FULL DEPTH AND WIDTH OF ALL UTILITY TRENCHES TO A MINIMUM OF 1'-0" ABOVE THE BOTTOM OF THE FOUNDATION AT THE EXTERIOR FACE OF THE BUILDING FOUNDATION.
5. CONDUITS SHALL NOT BE PLACED IN THE CONCRETE SLAB. CONDUITS SHALL BE PLACED IN THE SELECT FILL MATERIAL BENEATH THE VAPOR BARRIER. ALL PENETRATIONS OF THE VAPOR RETARDER SHALL BE SEALED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND REQUIREMENTS.
6. FOUNDATION WALL BACKFILL SHALL NOT BE UNBALANCED BY MORE THAN TWO (2) FEET ON EITHER SIDE AT ANY TIME. BASEMENT WALL AND RESTRAINED RETAINING WALL BACKFILL SHALL NOT BE PLACED UNLESS THE WALL IS ADEQUATELY BRACED. RETAINING WALL AND BASEMENT WALL BACKFILL SHALL BE FREE DRAINING GRANULAR BACKFILL ACCEPTABLE TO THE GEOTECHNICAL ENGINEER.
7. CONTRACTOR SHALL HAND TAMP BOTTOM OF EXCAVATIONS TO A HARD SURFACE BEFORE PLACING REINFORCING STEEL.
8. FOUNDATION CONTRACTOR TO ENSURE PROPER ANCHOR BOLT PROJECTION AND THAT ANCHOR BOLTS ARE HELD SECURELY IN POSITION PRIOR TO CONCRETE PLACEMENT. STRUCTURAL STEEL COLUMN ANCHOR BOLTS SHALL BE SET WITH A TEMPLATE.
9. SOIL BEARING TESTS SHALL BE PERFORMED BY A GEOTECHNICAL ENGINEER PRIOR TO INSTALLING FOOTINGS TO ENSURE DESIGN ALLOWABLE BEARING VALUES ARE MET. IF ACTUAL SITE CONDITIONS DO NOT SATISFY THESE REQUIREMENTS, COORDINATE ADJUSTMENTS WITH ARCHITECT/ENGINEER/GEOTECHNICAL ENGINEER.
10. ALL FOOTINGS SHALL BE CONSOLIDATED WITH A CONCRETE VIBRATOR AS PER THE REQUIREMENTS OF ACI 318 AND ACI 308R, LATEST EDITION.
11. FOOTINGS SHALL BE POURED IMMEDIATELY UPON COMPLETION OF EXCAVATION (I.E., NOT ALLOWED TO REMAIN OPEN) AND CLEANING OF FOOTING BEARING SURFACE. ALL SPOILS FROM THE FOOTING EXCAVATIONS SHALL BE REMOVED FROM THE BUILDING PAD.

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL DESIGN, DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO LOAD RESISTANCE FACTOR DESIGN (LRFD) ACCORDING TO THE LATEST AISC SPECIFICATION.
2. ALL WELDING SHALL CONFORM TO THE STANDARDS OF THE LATEST EDITION OF THE STEEL CONSTRUCTION MANUAL OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AND THE AMERICAN WELDING SOCIETY ANSI/AWS D1.1 STRUCTURAL WELDING CODE-STEEL. WELDING OF REINFORCING BARS SHALL COMPLY TO THE AMERICAN WELDING SOCIETY AWS D.1.4. SHORT CIRCUIT TRANSFER FOR THE GAS METAL ARC WELDING PROCESS IS NOT PERMITTED.
3. ELECTRODES FOR ALL FIELD AND SHOP WELDING SHALL BE CLASS E70XX. ELECTRODES FOR MOMENT CONNECTIONS SHALL BE CLASS E7018 WITH A CHARPY TOUGHNESS OF AT LEAST 20 FT-LBS AT -20 DEGREES FAHRENHEIT.
4. ALL STRUCTURAL STEEL ROLLED SHAPES SHALL CONFORM TO ASTM A992, AND ALL ANGLES, BARS, CHANNELS AND PLATES SHALL CONFORM TO ASTM A36. ALL SQUARE AND RECTANGULAR TUBE (Fy 46KSI) SHALL CONFORM TO ASTM A500 GRADE B AND ROUND PIPES (Fy 36KSI) SHALL CONFORM TO ASTM A53 GR B. ALL COLD-FORMED GIRTS AND PURLINS SHALL CONFORM TO ASTM A570M GR. 55.
5. HEADED ANCHOR RODS: ASTM F1554 GRADE 36 OR GRADE 55 ANCHOR AND HEAVY HEX NUT, AS INDICATED IN DETAILS.
6. ALL STRUCTURAL STEEL DETAILS AND CONNECTIONS SHALL CONFORM TO STANDARDS OF AISC. DOUBLE CONNECTIONS THROUGH COLUMN WEBS, BEAMS THAT FRAME OVER THE TOP OF COLUMNS, AND BEAM TO BEAM CONNECTIONS SHALL HAVE A BEAM ERECTION SEAT OR A STAGGERED CONNECTION WITH AT LEAST ONE INSTALLED BOLT REMAINING IN PLACE TO SUPPORT THE FIRST BEAM WHILE THE SECOND BEAM IS BEING ERECTED.
7. TYPICAL CONNECTIONS ARE INDICATED ON THE DRAWINGS.
8. THE DESIGN OF ALL STEEL CONNECTIONS NOT DETAILED ON THE DRAWINGS SHALL BE PERFORMED UNDER THE DIRECT SUPERVISION OF A REGISTERED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED, EMPLOYED BY THE FABRICATOR. CALCULATIONS SEALED BY THE FABRICATOR'S PROFESSIONAL ENGINEER MUST BE SUBMITTED IF REQUESTED.
9. IT IS THE INTENTION OF THE PLANS AND SPECIFICATIONS THAT SHOP CONNECTIONS BE WELDED OR BOLTED AND THAT FIELD CONNECTIONS BE BOLTED, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
10. ALL TYPICAL BEAM SIMPLE CONNECTIONS SHALL BE STANDARD DOUBLE ANGLE OR SINGLE ANGLE FRAMED BEAM CONNECTIONS. SHEAR TAB CONNECTIONS MAY BE USED AT LOCATIONS WHERE DOUBLE ANGLE CONNECTIONS ARE NOT POSSIBLE. SEATED BEAM CONNECTIONS SHALL NOT BE USED UNLESS INDICATED ON THE DRAWINGS. PROVIDE FULL DEPTH SHEAR TAB IF BEAM FRAMES ON ONLY ONE SIDE OF A GIRDER.
11. BEAM REACTIONS SHALL BE AS FOLLOWS UNLESS NOTED ON DRAWINGS: BEAM REACTIONS USED SHALL BE ONE-HALF THE TOTAL ALLOWABLE UNIFORM LOAD GIVEN IN TABLE 3-6 THROUGH 3-9 IN PART 3 OF THE AISC STEEL CONSTRUCTION MANUAL. ADD TO THE REACTION LISTED ABOVE, ANY LOADS OR REACTIONS OF MEMBERS SUPPORTED BY THE BEAM WITHIN THREE FEET OF BEAM END AND THE VERTICAL COMPONENTS OF FORCES IN BRACE MEMBERS FRAMING INTO THE BEAMS. BEAM REACTIONS FOR COMPOSITE BEAMS SHALL HAVE THE STANDARD AISC CAPACITY INCREASED BY 35 PERCENT.
12. ALL MISCELLANEOUS WELDS (FIELD OR SHOP) SHALL BE MINIMUM SIZE FILLET ALL AROUND IN ACCORDANCE WITH AISC 360. WELDING OF CONTINUOUS MEMBERS SHALL BE A MINIMUM OF 2 INCHES OF 3/16 INCH FILLET STITCH WELDS AT 12 INCHES O.C., STAGGERED EACH SIDE, UNLESS OTHERWISE NOTED. COLUMN BASE PLATES, CAP PLATES AND STIFFENER PLATES SHALL BE WELDED ALL AROUND.
13. PROVIDE ALL NECESSARY HOLES IN MISCELLANEOUS STRUCTURAL STEEL MEMBERS FOR ATTACHMENT OF NON-STRUCTURAL ITEMS (IE: HOLES FOR WINDOW HEAD ANCHORS), SEE ARCHITECTURAL DRAWINGS FOR REQUIREMENTS.
14. SPLICING OF STRUCTURAL STEEL MEMBERS WHERE NOT DETAILED IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
15. ALL CONNECTION BOLTS FOR STRUCTURAL STEEL MEMBERS SHALL CONFORM TO ASTM F3125, GRADE A325, TYPE 1 EXCEPT WHERE NOTED OTHERWISE. MINIMUM SIZE SHALL BE 3/4 INCH DIAMETER UNLESS NOTED OTHERWISE. BOLTS SHALL BE DIRECT TENSION INDICATING BOLTS CONFORMING TO ASTM F3125, GRADE F1852 WITH HARDENED WASHERS UNDER THE NUT AND SACRIFICIAL SPLINES. HEX NUTS SHALL CONFORM TO ASTM A563 AND WASHERS SHALL CONFORM TO ASTM F436.
16. SHOP BOLTED CONNECTIONS ARE PERMISSIBLE IF SUFFICIENT BOLT CLEARANCE IS AVAILABLE FOR TIGHTENING OF HIGH STRENGTH BOLTS. CLEARANCES SHALL BE IN ACCORDANCE WITH TABLE 7-15 AND 7-16 OF THE FIFTEENTH EDITION OF THE AISC STEEL CONSTRUCTION MANUAL. ALL STEEL MEMBERS AND ASSEMBLIES SHALL BE SHOP FABRICATED TO THE GREATEST EXTENT POSSIBLE. TRUSSES SHALL BE FULLY SHOP ASSEMBLED. FIELD SPLICES FOR SHIPPING SHALL ONLY BE AS APPROVED BY THE ENGINEER OF RECORD. THE STEEL FABRICATOR AND THE STEEL ERECTOR SHALL COORDINATE THE SHOP FABRICATION, SHIPPING AND ERECTION OF ALL STRUCTURAL MEMBERS AND ASSEMBLIES.
17. ALL GROUT USED UNDER STEEL COLUMN BASE PLATES SHALL BE OF NON-SHRINKABLE TYPE CONFORMING TO ASTM C1090 AND THE CORPS OF ENGINEERS SPECIFICATION CRD-C-621 AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 6000 PSI. 100 PERCENT OF VOID UNDER ALL BASE PLATES IS TO BE GROUTED. ALL BASE PLATES WITH A DIMENSION GREATER THAN 24" SHALL HAVE TWO 1" DIAMETER GROUT HOLES. IF THE SPACE UNDER A COLUMN BASE PLATE IS LESS THAN 1/4", A PRESSURE INJECTION SYSTEM SHALL BE USED.
18. ROOF SYSTEM OVER COLD FORMED METAL JOISTS SHALL BE RIGID INSULATION BOARD ON 1 ½" DEEP, 20 GAGE, TYPE B GALVANIZED (CONFORMING TO ASTM A924-94, WITH MINIMUM COATING CLASS OF G90 AS DEFINED IN ASTM A653-94) DECK FROM COLD ROLLED STEEL CONFORMING TO ASTM A653-99 OR ASTM A611 WITH Fy=50 KSI, AND HAVING A MINIMUM MOMENT OF INERTIA OF 0.217 INCH TO THE FOURTH PER FOOT OF WIDTH. SIDELAPS SHALL BE FASTENED WITH #10 TEKS AT THIRD POINTS OR 1'-0" ON CENTER MAX. (3 MIN. PER SPAN). DECK ATTACHMENT SHALL BE HILTI X-EDNK22 THQ12 (1/8"-1/4" INCLUSIVE), X-EDN19 THQ12 (3/16"-3/8" INCLUSIVE), OR X-ENP19 (1/4" OR THICKER) AS RECOMMENDED BY THE MANUFACTURER OR WELD DECK THROUGH 5/8" DIAMETER PUDDLE WELDS TO SUPPORTING MEMBERS AT 1'-0" ON CENTER AT END LAPS AND AT INTERMEDIATE SUPPORTS. AT SPANDREL BEAMS OR DECK SUPPORT ANGLES AND FOR A 10'-0" SQUARE AREA AT CORNERS WELD DECK TO ALL SUPPORTS AT 6" ON CENTER.
19. ROOF DECK SHALL BE 3" DEEP, 18 GAGE, TYPE 3N DECK CONFORMING TO ASTM A653 WITH Fy=40 KSI. DECK SHALL BE GALVANIZED, CONFORMING TO ASTM A924, WITH A MINIMUM ZINC COATING CLASS OF G90 (LW FILL) OR G60 (RIGID BOARD). DECK SHALL HAVE A MINIMUM MOMENT OF INERTIA OF 1.123 INCH TO THE FOURTH PER FOOT OF WIDTH. FASTEN SIDELAPS WITH #10 TEK SCRES; ONE AT MIDSPAN OR 3'-0" ON CENTER MAX. WELD DECK THROUGH 5/8" DIAMETER PUDDLE WELDS (OR EQUIVALENT SCREWS) TO EACH STRUCTURAL SUPPORTING MEMBER AT 1'-4" ON CENTER AT END LAPS AND AT INTERMEDIATE SUPPORTS. AT SPANDREL BEAMS AND DECK SUPPORT ANGLES, AND FOR A 10'-0" SQUARE AREA AT ROOF CORNERS, THE DECK SHALL BE WELDED TO ALL SUPPORTS AT 8" ON CENTER.
20. FLOOR DECK AT THE FIRST FLOOR SHALL BE CORRUGATED DECK CONFORMING TO ASTM A653 WITH Fy=60 KSI. DECK SHALL BE GALVANIZED, CONFORMING TO ASTM A924, WITH A MINIMUM ZINC COATING CLASS OF G60. DECK SHALL BE 20 GAGE METAL FORMS, 1 1/2" DEEP AND SHALL HAVE A MINIMUM MOMENT OF INERTIA OF 0.217 INCH TO THE FOURTH PER FOOT OF WIDTH. WELD DECK THROUGH WELD WASHERS TO EACH STRUCTURAL SUPPORTING MEMBER AT EVERY OTHER CORRUGATION AT END LAPS, AND 2'-0" ON CENTER AT INTERMEDIATE SUPPORTS. AT SPANDREL BEAMS AND DECK SUPPORT ANGLES, THE DECK SHALL BE WELDED TO ALL SUPPORTS AT 6" ON CENTER. THE SLAB SHALL BE 5 ½" NORMAL WEIGHT CONCRETE REINFORCED WITH 4x4-W2.9xW2.9 WELDED WIRE FABRIC LAPPED ONE MESH AT SPLICES.
21. FLOOR DECK AT THE PENTHOUSE SHALL BE CORRUGATED DECK CONFORMING TO ASTM A653 WITH Fy=60 KSI. DECK SHALL BE GALVANIZED, CONFORMING TO ASTM A924, WITH A MINIMUM ZINC COATING CLASS OF G60. DECK SHALL BE 20 GAGE METAL FORMS, 5/8" DEEP AND SHALL HAVE A MINIMUM MOMENT OF INERTIA OF 0.023 INCH TO THE FOURTH PER FOOT OF WIDTH. WELD DECK THROUGH WELD WASHERS TO EACH STRUCTURAL SUPPORTING MEMBER AT EVERY OTHER CORRUGATION AT END LAPS, AND 2'-0" ON CENTER AT INTERMEDIATE SUPPORTS. AT SPANDREL BEAMS AND DECK SUPPORT ANGLES, THE DECK SHALL BE WELDED TO ALL SUPPORTS AT 6" ON CENTER. THE SLAB SHALL BE 3 ½" NORMAL WEIGHT CONCRETE REINFORCED WITH 4x4-W2.9xW2.9 WELDED WIRE FABRIC LAPPED ONE MESH AT SPLICES.
22. STEEL DECK SHALL ALWAYS BE INSTALLED WITH DIRECTION OF FLUTES PERPENDICULAR TO STEEL FRAMING MEMBERS. DECK SHALL BE CUT TO INSURE A MINIMUM OF THREE SPANS PER DECK WIDTH.
23. ALL STRUCTURAL STEEL WHICH IS OUTSIDE THE BUILDING ENVELOPE SHALL BE HOT DIPPED GALVANIZED. ZINC COATING SHALL MEET THE REQUIREMENTS OF ASTM 123-73, WITH A MINIMUM COATING GRADE OF GR60 AND SHALL BE APPLIED AFTER FABRICATION. ALL FIELD WELDS SHALL BE GROUND SMOOTH AND TOUCHED UP WITH ZINC RICH PAINT.
24. STEEL COLUMNS SHALL BE SPLICED A MINIMUM OF 4'-0" ABOVE THE FINISH FLOOR IN STORIES WHERE SPLICES OCCUR. COLUMNS SHALL BE SPLICED EVERY TWO LEVELS. COLUMNS SHALL HAVE HOLES FOR ¾" DIAMETER SAFETY CABLES OR PLATES WITH A HOLE WELDED TO THE COLUMN. PROVIDE AN L3x3x1/4" DECK SUPPORT ANGLE ON ALL SIDES OF THE COLUMN.
25. THE GENERAL CONTRACTOR AND THEIR SUBCONTRACTOR'S SHALL COMPLY TO OSHA 29 CFR 1926 SUBPART R, SAFETY STANDARDS FOR STEEL ERECTION.
26. AS SCOPE AND PERFORMANCE DOCUMENTS, THE DRAWINGS AND SPECIFICATIONS DO NOT INDICATE OR DESCRIBE ALL OF THE WORK REQUIRED FOR THE PERFORMANCE AND COMPLETION OF THIS WORK. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE FABRICATION AND INSTALLATION OF ALL MISCELLANEOUS METAL ITEMS INDICATED, DESCRIBED, OR IMPLIED ON THE STRUCTURAL AND/OR THE ARCHITECTURAL DRAWINGS. MISCELLANEOUS STEEL ITEMS, WITHIN AN ASSEMBLY AND NOT ATTACHED TO THE STRUCTURE, ARE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND THEIR SUBCONTRACTORS WHETHER THEY ARE SHOWN OR NOT SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS. SUCH ASSEMBLIES INCLUDE BUT ARE NOT LIMITED TO, EXTERIOR AND INTERIOR WALL ASSEMBLIES, CEILING ASSEMBLIES, PARTITION ASSEMBLIES, SHELF AND CABINET ASSEMBLIES AND ALL OTHER SIMILAR ASSEMBLIES. ANY MISCELLANEOUS METAL ITEMS INDICATED ON THE ARCHITECTURAL DRAWINGS AND NOT SHOWN ON STRUCTURAL DRAWINGS SHALL BE A MINIMUM OF L4x4x1/2", C7x9.8, 3/8" PLATE OR HSS4x4x3/8" UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER.

COLD ROLLED STEEL SPECIFICATIONS (SSMA)

1. ALL STRUCTURAL MEMBERS AND CONNECTIONS SHALL BE DESIGNED IN ACCORDANCE WITH AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS." LATEST EDITION.
2. ALL STUDS AND/OR JOISTS AND ACCESSORIES SHALL BE OF THE TYPE, SIZE, GAUGE, AND SPACING SHOWN ON THE DRAWINGS.
3. ALL STUDS, RUNNERS, JOISTS, AND TRUSSES SHALL BE FORMED FROM GALVANIZED STEEL, CORRESPONDING TO THE REQUIREMENTS OF ASTM A446, WITH A MINIMUM YIELD STRENGTH OF 50 KSI FOR 0.097, 0.068, 0.054 THICK MEMBERS AND 33 KSI FOR 0.043 AND 0.033 THICK MEMBERS AND FLAT STRAP BRACING.
4. CONTRACTOR SHALL PROVIDE AND INSTALL ALL NECESSARY BRACING FOR COLD-FORMED TRUSSES.
5. PLANS AND DETAILS FOR FRAMING ARE SCHEMATIC REPRESENTATION OF THE FRAMING AT VARIOUS LOCATIONS AND CONDITIONS ON THIS PROJECT. THE CONTRACTOR SHALL NOT SCALE OR COUNT FRAMING MEMBERS SHOWN AS A SUBSTITUTE FOR SHOP DRAWINGS AND AN ACCURATE QUANTITY TAKEOFF. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL FRAMING NECESSARY TO COMPLETELY FRAME THE PROJECT AND PROVIDE FOR ALL CONDITIONS SHOWN ON THE ARCHITECTURAL DRAWINGS.
6. PROVIDE TRIPLE STUDS AT ALL CORNERS AND AT ALL BEAM BEARINGS THROUGH TO THE FOUNDATION UNLESS NOTED OTHERWISE.
7. PRIOR TO FABRICATION THE CONTRACTOR SHALL SUBMIT ERECTION DRAWINGS TO THE STRUCTURAL ENGINEER FOR APPROVAL.
8. PREFABRICATED PANELS SHALL BE SQUARE, WITH COMPONENTS ATTACHED IN A MANNER AS TO PREVENT RACKING. HANDLING AND LIFTING SHALL BE DONE IN A MANNER SO AS NOT TO CAUSE DISTORTION IN ANY MANNER.
9. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR, AS REQUIRED, FOR AN ANGULAR FIT AGAINST ABUTTING MEMBERS.
10. AXIALLY LOADED STUDS SHALL BE INSTALLED IN A MANNER WHICH WILL ASSURE THAT THEIR ENDS ARE POSITIONED AGAINST THE INSIDE OF TRACK WEB PRIOR TO FASTENING.
11. FASTENING OF COMPONENTS SHALL BE SELF-DRILLING SCREWS OR WELDS. SCREWS OR WELDS SHALL BE OF SUFFICIENT SIZE TO ENSURE THE STRENGTH OF THE CONNECTION. WIRE TYING OF COMPONENTS SHALL NOT BE PERMITTED. ALL WELDS SHALL BE TOUCHED UP WITH A ZINC-RICH PAINT.
12. RUNNERS SHALL BE SECURELY ANCHORED TO THE SUPPORTING STRUCTURE. PROPOSED CONNECTION TO BE SUBMITTED FOR APPROVAL.
13. ABUTTING LENGTHS OF TRACK SHALL EACH BE SECURELY ANCHORED TO A COMMON STRUCTURAL ELEMENT, BUTT-WELDED, OR SPLICED.
14. STUDS SHALL BE PLUMB, ALIGNED AND SECURELY ATTACHED TO FLANGES OF BOTH UPPER AND LOWER TRACKS.
15. JACK STUDS OR CRIPPLES SHALL BE INSTALLED BELOW WINDOW SILLS, ABOVE WINDOW AND DOOR HEADERS, AND WHERE NEEDED TO FURNISH SUPPORT, AND SHALL BE SECURELY ATTACHED TO CONNECTING MEMBERS.
16. RESISTANCE TO MINOR AXIS BENDING AND ROTATION SHALL BE PROVIDED BY GYPSUM BOARD OR GYPSUM SHEATHING AND HORIZONTAL STRAP AND BLOCKING OR COLD-ROLLED CHANNEL BRACING AT THIRD POINTS.
17. SPLICES IN AXIALLY LOADED STUDS SHALL NOT BE PERMITTED.
18. PROVIDE A MINIMUM OF (3) #12 SCREWS FOR ALL STUD TO STUD CONNECTIONS.
19. BRIDGING SHALL BE INSTALLED IMMEDIATELY AFTER JOISTS ARE ERECTED AND BEFORE CONSTRUCTION LOADS ARE APPLIED TO PREVENT FLANGES IN COMPRESSION. BRIDGING SHALL CONSIST OF SOLID BLOCKING PLUS STRAP BRACING OR 1 1/2" COLD-ROLLED CHANNELS SCREW-ATTACHED OR WELDED TO BOTTOM JOIST FLANGES. BRIDGING SHALL BE INSTALLED AT MID SPAN FOR SPANS 16'-0" OR LESS AND AT 6'-0" O.C. MAX FOR SPANS GREATER THAN 16'-0" U.N.O. SOLID BLOCKING, OF FIELD-CUT TRACK OR JOIST SECTION, SHALL BE PROVIDED, WELDED, OR SCREW-ATTACHED BETWEEN OUTER JOISTS, OVER ALL INTERIOR SUPPORTS AND ADJACENT TO OPENINGS AT 10'-0" O.C. MAX. COLD-ROLLED CHANNELS OR STRAP BRACING OF 1 1/2 X 33 MIL (0.033") CORROSION-RESISTANT STEEL SHALL BE SCREW-ATTACHED TO BOTTOM JOIST FLANGE BETWEEN SOLID BLOCKING. REFERENCE MANUFACTURER INSTALLATION INSTRUCTIONS.
20. PROVIDE HORIZONTAL BRIDGING IN ALL STUD SYSTEMS AT FOUR FEET ON CENTER VERTICALLY FOR AXIALLY LOADED STUDS AND AT SIX FEET ON CENTER FOR NON-LOAD BEARING STUDS UNLESS NOTED OTHERWISE. WELD AT EACH STUD INTERSECTION.
21. SSMA STEEL FRAMING CARRIES A FOUR PART CODE THAT IDENTIFIES THE WEB SIZE, STYLE, FLANGE WIDTH, AND STEEL THICKNESS:

362 S 162-43
WEB SIZE: / STYLE / FLANGE WIDTH / STEEL THICKNESS (MIL)

WEB SIZE		STYLE	FLANGE WIDTH	STEEL THICKNESS (MIL)
250-2 1/2"	600-6"	S-STUD	125-1 1/4"	33
350-3 1/2"	800-8"	J-JOIST	137-1 3/8"	43
362-3 5/8"	1000-10"	T-TRACK	162-1 5/8"	54
400-4"	1200-12"		200-2"	68
550-5 1/2"	1400-14"		250-2 1/2"	97

MASONRY

1. ALL CONCRETE MASONRY UNITS SHALL BE HOLLOW LOAD BEARING UNITS CONFORMING TO THE REQUIREMENTS OF ASTM C90, TYPE 1 AND THE QUALITY CONTROL STANDARDS OF THE CONCRETE MASONRY ASSOCIATION.
2. ALL CONCRETE MASONRY SHALL HAVE LIGHTWEIGHT MASONRY UNITS WITH A DRY DENSITY OF NOT MORE THAN 105 POUNDS PER CUBIC FOOT.
3. ALL MASONRY UNITS SHALL HAVE A MINIMUM NET COMPRESSIVE STRENGTH OF 2500 PSI AND A MINIMUM NET TENSILE STRENGTH OF NOT LESS THAN 125 PSI, WHEN TESTED IN ACCORDANCE WITH THE METHODS SET FORTH IN THE QUALITY CONTROL STANDARDS OF THE CONCRETE MASONRY ASSOCIATION.
4. MASONRY UNITS SHALL HAVE CURED FOR NOT LESS THAN 28 DAYS WHEN PLACED IN THE STRUCTURE.
5. ALL MASONRY UNITS SHALL HAVE A MAXIMUM LINEAR SHRINKAGE OF .065 OF 1% FROM THE SATURATED TO THE OVEN DRY CONDITION, WHEN TESTED IN ACCORDANCE WITH THE METHODS SET FORTH IN THE QUALITY CONTROL STANDARDS OF THE CONCRETE MASONRY ASSOCIATION.
6. MORTAR SHALL BE FRESHLY PREPARED AND UNIFORMLY MIXED IN THE RATIO OF 1 PART PORTLAND CEMENT, ¼ PART MINIMUM TO ½ PART MAXIMUM LIME PUTTY OR HYDRATED LIME, DAMP LOOSE SAND NOT LESS THAN 2-1/4 AND NOT MORE THAN 3 TIMES THE SUM OF THE VOLUMES OF THE CEMENT AND LIME USED, AND SHALL CONFORM TO ASTM C270, TYPE 'S'.
7. GROUT FOR POURING SHALL BE OF FLUID CONSISTENCY AND MIXED IN THE RATIO BY VOLUMES, 1 PART PORTLAND CEMENT, 2 ¼ PARTS MINIMUM TO 3 PARTS MAXIMUM DAMP LOOSE SAND, 1 PART MINIMUM TO 2 PARTS MAXIMUM PEA GRAVEL, AND 0 TO 1/10 PART MAXIMUM HYDRATED LIME. MIX SHALL CONFORM TO ASTM C 476 WITH A 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI. MAXIMUM GROUT HEIGHT SHALL BE 4'-0".
8. GROUT FOR PUMPING SHALL BE OF FLUID CONSISTENCY AND SHALL HAVE NOT LESS THAN 7 SACKS OF CEMENT IN EACH CUBIC YARD OF GROUT. THE MIX SHALL BE SUBSTITUTED FOR APPROVAL.
9. THE COMPRESSIVE STRENGTH OF THE MASONRY (f'm) SHALL BE 1900 PSI.
10. ALL CELLS WITH REINFORCING BARS SHALL BE GROUTED SOLID.
11. ALL CELLS SHOWN TO HAVE DRILLED EXPANSION ANCHORS, EMBEDDED HEADED STUDS OR OTHER EMBEDDED ANCHORS SHALL BE GROUTED SOLID.
12. HORIZONTAL JOINT REINFORCEMENT SPACED AT 16" O.C. MAX. VERTICALLY SHALL CONFORM TO ASTM A951 WITH A MINIMUM YIELD STRENGTH OF 70,000 PSI AND A MINIMUM SIZE OF 9 GAGE FOR SIDE RODS AND 9 GAGE FOR TRUSS RODS.
13. OPENINGS IN MASONRY WALL SHALL HAVE EITHER MASONRY OR STEEL LINTELS AS DETAILED ON THE DRAWINGS. WHEN NO LINTEL IS DETAILED A MINIMUM OF 2-#4 BARS IN A SOLID GROUTED LINTEL BLOCK SHALL BE INSTALLED. THE BARS SHALL EXTEND A MINIMUM OF EIGHT INCHES BEYOND THE EDGE OF THE OPENING AND THE JAMB AT EACH SIDE OF THE OPENING SHALL BE GROUTED SOLID FOR A DISTANCE OF EIGHT INCHES WITH A #5 VERTICAL MINIMUM AT EACH JAMB. LAP BARS 2'-0" MIN. OR 40 BAR DIAMETERS AT SPLICES, INTERSECTIONS AND CORNERS. STEEL LINTELS SHALL BEAR 8" MINIMUM AT EACH END ON FLASHING ABOVE AND BELOW THE ANGLE. VERTICAL CONTROL JOINTS SHALL EXTEND UP FROM THE END OF THE STEEL LINTEL, UNLESS 15# FELT OR FLASHING IS PROVIDED TOP AND BOTTOM OF LINTEL ANGLE WHERE ANGLE BEARS ON BRICK.
14. LINTEL BLOCKS SHALL BE "U" SHAPED UNITS WITH SOLID BOTTOMS AND ARE TO BE USED OVER WINDOW AND DOOR OPENINGS. BOND BEAM BLOCKS SHALL BE OPEN BOTTOM UNITS AND ARE TO BE USED AT THE TOPS OF WALLS AND AT THE MID-HEIGHT OF WALL OR AT 8'-0" ON CENTER VERTICALLY MAXIMUM UNLESS NOTED OTHERWISE ON THE DRAWINGS. PROVIDE 2-#4 BARS IN A SOLID GROUTED BOND BEAM UNLESS NOTED OTHERWISE. LINTEL BLOCKS SHALL NOT BE USED IN PLACE OF BOND BEAM BLOCKS.
15. ALL MASONRY TIES TO BACKUP STRUCTURE SHALL BE HOT DIPPED GALVANIZED. PROVIDE A HECKMANN NO. 315 ANCHOR WITH NO. 316 TRIANGULAR TIE ON COLUMNS AT 16" (15" AT KING SIZE BRICK) ON CENTER VERTICALLY AND A HECKMANN NO. 191 OR 192 ANCHOR ON EACH SIDE ALL BEAMS AT 16" ON CENTER HORIZONTALLY OR APPROVED EQUAL UNLESS NOTED OTHERWISE ON THE DRAWINGS. MASONRY TIES TO WALL STUDS SHALL BE A HECKMANN NO. 316 TRIANGULAR TIE WITH A HECKMANN NO. 315-C SCREW ON ANCHOR STRAP OR HECKMANN #77 WING NUT POS-I-TIE ANCHOR SPACED 16" (15" AT KING SIZE BRICK) ON CENTER HORIZONTALLY AND 16" ON CENTER VERTICALLY OR APPROVED EQUAL. AT ALL CORNERS AND INTERSECTIONS PROVIDE TWO VERTICAL ROWS OF ANCHORS SPACED 16" APART AND 16" ON CENTER VERTICALLY. TRIANGULAR TIES SHALL EXTEND ¾" FROM FACE OF MASONRY. ANCHOR STRAPS SHALL BE ATTACHED TO METAL STUDS WITH TWO (2) #10-16x 1 ½" CADMIUM PLATED HEX HEAD SHEET METAL SCREWS WITH NEOPRENE WASHER.
16. MASONRY WALLS SHALL HAVE VERTICAL CONTROL JOINTS AT APPROXIMATELY SIXTEEN (16) FEET ON CENTER AND FOUR (4) FEET MAXIMUM FROM CORNERS. COORDINATE THE LOCATION OF JOINTS WITH THE ARCHITECT. PROVIDE HECKMANN NO. 351 CONTROL JOINT ANCHORS AT 16" ON CENTER VERTICALLY AT BRICK MASONRY AND HECKMANN NO. 350 CONTROL JOINT ANCHORS AT 16" ON CENTER VERTICALLY AT CONCRETE MASONRY UNITS OR APPROVED EQUAL.
17. AT FREE VERTICAL EDGES OF WALLS PROVIDE 1-#5 VERTICAL IN GROUT FILLED END CORE. UNLESS NOTED OTHERWISE ON THE DRAWINGS.
18. PROVIDE A MINIMUM OF #4 AT 48" ON CENTER VERTICAL WALL REINFORCING AND DOWELS IN FULLY GROUTED CELLS AT ALL EXTERIOR AND INTERIOR WALLS UNLESS A GREATER REINFORCING IS SHOWN ON THE PLANS OR IN THE DETAILS. PROVIDE A ½" DIAMETER DEFORMED BAR ANCHORS AT 48" ON CENTER WELDED TO STRUCTURAL MEMBERS SUPPORTING MASONRY ABOVE UNLESS NOTED OTHERWISE ON THE DRAWINGS.
19. PROVIDE HOHMANN AND BARNARD RB-8 (OR EQUAL) REBAR POSITIONERS AT EVERY THIRD COURSE AND AT SPLICE LOCATIONS.
20. ALL MASONRY DESIGN IS BASED ON CHAPTER 21 OF INTERNATIONAL BUILDING CODE, LATEST EDITION AND ACI 530, LATEST EDITION.

FRP (FIBER REINFORCED POLYMER) REINFORCEMENT

1. FIBER REINFORCED POLYMER (FRP) COMPOSITE STRENGTHENING SHALL BE DESIGNED BY A FRP STRENGTHENING ENGINEER WORKING FOR THE REPAIR CONTRACTOR.
2. FRP STRENGTHENING SHALL SATISFY THE DESIGN REQUIREMENTS INDICATED ON PROJECT DRAWINGS. THE FRP ENGINEER SHALL PROVIDE RESULTS OF ANALYSIS BASED ON THE SPECIFIED LOADING FOR NEW EQUIPMENT. FRP SHALL BE DESIGNED TO SATISFY THE DESIGN FORCES RESULTING FROM THE ADDITIONAL LOADING INDICATED ON THE STRUCTURAL DOCUMENTS.
3. SUBMIT FOR REVIEW AND APPROVAL DESIGN DETAILS AND CALCULATIONS PREPARED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED.
4. DESIGN OF FRP STRENGTHENING SHALL BE BASED ON THE FOLLOWING PERFORMANCE REQUIREMENTS:
   - 4.1. DESIGN OF THE FRP REINFORCEMENT SHALL BE IN ACCORDANCE WITH ACI 440.2R-17.
   - 4.2. MINIMUM FRP DESIGN PROPERTIES (LAMINATE PROPERTIES – USE OF DRY FIBER PROPERTIES IS NOT ALLOWED):
     - 4.3.1. TENSILE STRENGTH: 155 KSI
     - 4.3.2. MODULUS OF ELASTICITY: 14,000 KSI
     - 4.3.3. ELONGATION AT BREAK: 1.1%
5. REQUIRED SUBMITTALS FOR FRP:
   - 5.1. PRODUCT DATA SHEETS
   - 5.2. CURRENT ICC-ES LISTING REPORT
   - 5.3. QUALITY CONTROL INSPECTION AND TESTING PROGRAM
   - 5.4. A LIST OF 15 FRP STRENGTHENING PROJECTS DESIGNED BY THE FRP ENGINEER WITH THE MANUFACTURER'S FRP COMPOSITE SYSTEM.
   - 5.5. A LIST OF 15 FRP STRENGTHENING PROJECTS COMPLETED BY THE FRP CONTRACTOR WITH THE MANUFACTURER'S FRP COMPOSITE SYSTEM IN THE PAST 2 YEARS. THE LIST SHOULD INCLUDE PROJECT NAME, DATE OF WORK, AMOUNT OF FRP AND TYPE OF WORK PERFORMED.
6. INSTALL FRP SYSTEM FOLLOWING THE PROCEDURES BY THE FRP MANUFACTURER.
7. FRP FABRIC SHALL BE SATURATED USING A SATURATING MACHINE. MANUAL SATURATION OF FRP FABRIC SHALL NOT BE PERMITTED.
8. FRP SYSTEMS SHALL BE INSTALLED BY AN EXPERIENCED AND TRAINED APPLICATOR CERTIFIED BY THE MANUFACTURER OF THE FRP SYSTEM.
9. FRP LAYOUT VARIATIONS TO ADDRESS FIELD CONDITIONS SHALL BE COORDINATED BY GENERAL CONTRACTOR WITH THE FRP DESIGNER TO ENSURE CONFORMANCE WITH THE DESIGN INTENT. SUBMIT AS-BUILT DRAWINGS INCLUDING FINAL LAYOUT OF THE STRENGTHENING SOLUTIONS AT THE COMPLETION OF THE PROJECT.
10. CONDUCT DIRECT TENSION ADHESION TESTING PER ASTM D7522 or ASTM D4541.
11. PROVIDE LABORATORY TESTING PER ASTM D3039 FOR EACH BATCH OF FIBER AND EPOXY USED ON THE PROJECT.
12. FRP STRENGTHENED ELEMENTS SHALL BE FIREPROOFED TO ACHIEVE A 2-HOUR RATING.
13. PLANS SHOWING FRP LOCATIONS ARE SCHEMATIC REPRESENTATIONS OF THE FRP SYSTEM. THIS DOES NOT REPRESENT THE ACTUAL AND/OR FINAL LOCATION OF THE FRP SYSTEM. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL FRP STRENGTHENING TO COMPLETE THE PROJECT.
14. THE FRP DESIGN DRAWINGS SHALL INDICATE THE FOLLOWING:
   - 14.1. LOCATION OF THE FRP SYSTEM RELATIVE TO THE EXISTING STRUCTURE.
   - 14.2. DIMENSIONS AND ORIENTATION OF EACH PLY, LAMINATE, OR NEAR-SURFACE-MOUNTED (NSM) BAR.
   - 14.3. NUMBER OF PLIES AND BARS AND THE SEQUENCE OF INSTALLATION.
   - 14.4. LOCATION OF SPLICES AND LAP LENGTH.
   - 14.5. MATERIAL PROPERTIES OF THE FRP LAMINATES.

FULLY SPRINKLERED

100% BID SET

No.	REVISION DESCRIPTION	DATE

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Drawing Title
STRUCTURAL GENERAL NOTES

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date	Checked	Drawn
12/21/2022	SJB	ZAF

Project Number: 589-704
Building Number: 26
Drawing Number: **SA001**
Drawing # 15 OF 190

Veterans Health Administration
VA U.S. Department of Veterans Affairs

12/19/2022 3:41:59 PM

VA FORM 08-6231

STRUCTURAL ABBREVIATIONS

+/-	PLUS OR MINUS
ADDNL	ADDITIONAL
ADJ	ADJACENT
AFF	ABOVE FINISH FLOOR
ALT	ALTERNATE
ARCH	ARCHITECT OR ARCHITECTURAL
B/W	BETWEEN
BLDG	BUILDING
BLKG	BLOCKING
BM	BEAM
BOD	BOTTOM OF DECK
BOS	BOTTOM OF STEEL
BP	BASE PLATE
CF	COLD FORMED
CFMF	COLD-FORMED METAL FRAMING
CIP	CAST IN PLACE
CJ	CONTROL JOINT
CL	CENTERLINE
CLR	CLEAR/CLEARANCE
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONT	CONTINUOUS
CTR	CENTER
db	DIA OF REINF BAR, DIA OF BOLT
DBA	DEFORMED BAR ANCHOR
DBL	DOUBLE
DIA	DIAMETER
DIAG	DIAGONAL
DWL	DOWEL
EA	EACH
EJ	EXPANSION JOINT
EL	ELEVATION
EOD	EDGE OF DECK
EOS	EDGE OF SLAB
EQ	EQUAL
EW	EACH WAY
EXIST	EXISTING
EXT	EXTERIOR
FLR	FLOOR
FND	FOUNDATION
FS	FAR SIDE
FTG	FOOTING
FV	FIELD VERIFY
GA	GAUGE
GALV	GALVANIZED
GC	GENERAL CONTRACTOR
HORIZ	HORIZONTAL
HSA	HEADED STUD ANCHOR
HSS	HOLLOW STRUCTURAL SECTION
IF	INSIDE FACE
INT	INTERIOR
JST	JOIST
K	KIPS (1000 LBS)
LCE	COMPRESSION EMBEDMENT LENGTH
LCS	COMPRESSION LAP SPLICE LENGTH
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LTE	TENSION EMBEDMENT LENGTH
LTS	TENSION LAP SPLICE LENGTH
LW	LIGHTWEIGHT
MANUF	MANUFACTURER
MTL	METAL
NIC	NOT IN CONTRACT
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OCEF	ON CENTER, EACH FACE
OCEW	ON CENTER, EACH WAY
OF	OUTSIDE FACE
OH	OPPOSITE HAND
OPP	OPPOSITE
PAF	POWDER ACTUATED FASTENER
PEMB	PRE-ENGINEERED METAL BUILDING
PL	PLATE
PLF	POUNDS PER LINEAL FOOT
PREFAB	PREFABRICATED
PRELIM	PRELIMINARY
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
RE: OR REF	REFER TO
REINF	REINFORCING
REQD	REQUIRED
SIM	SIMILAR
SLV	SHORT LEG VERTICAL
SOG	SLAB ON GRADE
SQ	SQUARE
SS	STAINLESS STEEL
STD	STANDARD
STL	STEEL
SW	SHEAR WALL
SYM	SYMMETRIC
T&B	TOP AND BOTTOM
TOC	TOP OF CONCRETE
TOD	TOP OF DECK
TOS	TOP OF STEEL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
VIF	VERIFY IN FIELD
W/	WITH
W/O	WITHOUT
WF	WIDE FLANGE
WP	WORK POINT
WWF	WELDED WIRE FABRIC

STRUCTURAL TESTING AND INSPECTIONS

SPECIAL INSPECTIONS

SPECIAL INSPECTION WORK AND THE FINAL LETTER OF COMPLIANCE HAVE NOT BEEN INCLUDED IN THE STRUCTURAL ENGINEER OF RECORD'S SCOPE OF SERVICES. PER IBC CHAPTER 17, THE OWNER IS RESPONSIBLE FOR OBTAINING THE SERVICES OF THE SPECIAL INSPECTOR AND THE TESTING LABORATORY. PER DIVISION 1 OF THE SPECIFICATIONS, THE OWNER IS REQUIRING THE CONTRACTOR TO INCLUDE THE COST OF THE SERVICES FOR THE SPECIAL INSPECTOR AND TESTING LABORATORY WITHIN THEIR SCOPE OF WORK, BUT ALL REPORTING OF TEST AND INSPECTION RESULTS SHALL BE DELIVERED DIRECTLY TO THE OWNER NOT THE CONTRACTOR. SPECIAL INSPECTIONS CAN BE PROVIDED BY AN INDEPENDENT SPECIAL INSPECTOR APPROVED BY THE BUILDING AUTHORITY OR BY THE ENGINEER OF RECORD. THE SPECIAL INSPECTION WORK DOES NOT INCLUDE THE TESTING LABORATORY SERVICES AS CALLED FOR ON THE DRAWINGS. ARRANGEMENTS FOR SPECIAL INSPECTIONS SHOULD BE MADE PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE OWNER IF SPECIAL INSPECTIONS ARE REQUIRED ON THE APPROVED PERMIT DRAWINGS AND FOR NOTIFYING THE TESTING LABORATORY AND SPECIAL INSPECTOR IN A TIMELY MANNER PRIOR TO PROCEEDING WITH CONSTRUCTION OPERATIONS. THE CONTRACTOR SHALL NOT PROCEED WITH ANY WORK REQUIRING INSPECTIONS WITHOUT THE TESTING LABORATORY'S OR SPECIAL INSPECTOR'S PRESENCE. THE STRUCTURAL ENGINEER WILL NOT PROVIDE A FINAL LETTER OF COMPLIANCE AFTER THE WORK IS COMPLETE UNLESS THEY HAVE PERFORMED THE SPECIAL INSPECTIONS.

STRUCTURAL STATEMENT OF SPECIAL INSPECTIONS

INCLUDED HERE ARE SPECIAL INSPECTIONS REQUIRED FOR THE STRUCTURAL ELEMENTS FOR THIS PROJECT. REFERENCE STATEMENT OF SPECIAL INSPECTIONS BY ARCHITECT AND OTHER CONSULTANTS FOR SPECIAL INSPECTION DETAILS OF NON-STRUCTURAL ITEMS REQUIRED BY IBC AND PROVIDED IN THE SPECIFICATIONS.

PER IBC 1704.2.4, THE STRUCTURAL SPECIAL INSPECTOR SHALL KEEP RECORDS OF ALL STRUCTURAL INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE OWNER AND THE STRUCTURAL REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE (SRDP). DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF SUCH DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER AND THE SRDP. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITIES.

A FINAL REPORT OF SPECIAL INSPECTION DOCUMENTING COMPLETION OF ALL SPECIAL INSPECTIONS, TESTING AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED TO THE OWNER AND THE SRDP PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY AND USE.

CONTRACTOR RESPONSIBILITY

PER IBC 1704.4, EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A MAIN WIND- OR SEISMIC FORCE-RESISTING SYSTEM, DESIGNATED SEISMIC SYSTEM OR A WIND- OR SEISMIC-RESISTING COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS.

STRUCTURAL SCHEDULE OF SPECIAL INSPECTIONS

QUALIFICATIONS OF INSPECTORS AND TESTING TECHNICIANS

THE QUALIFICATIONS OF ALL PERSONNEL PERFORMING SPECIAL INSPECTION AND TESTING ACTIVITIES ARE SUBJECT TO THE APPROVAL OF THE OWNER. THE CREDENTIALS OF ALL INSPECTORS AND TESTING TECHNICIANS SHALL BE PROVIDED TO THE SPECIAL INSPECTOR FOR THEIR RECORDS.

KEY FOR MINIMUM QUALIFICATION OF INSPECTION AGENTS

WHEN THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE OR SPECIAL INSPECTOR OF RECORD DEEMS APPROPRIATE THAT THE INDIVIDUAL PERFORMING THE STIPULATED TEST OR INSPECTION HAVE A SPECIFIC CERTIFICATION, LICENSE OR EXPERIENCE AS INDICATED BELOW. SUCH REQUIREMENT SHALL BE LISTED BELOW AND SHALL BE CLEARLY IDENTIFIED WITHIN THE SCHEDULE UNDER THE AGENT QUALIFICATION DESIGNATION.

- PE/SE STRUCTURAL ENGINEER – A LICENSED SE OR PE SPECIALIZING IN THE DESIGN OF BUILDING STRUCTURES
- PE/GE GEOTECHNICAL ENGINEER – A LICENSED PE SPECIALIZING IN SOIL MECHANICS AND FOUNDATIONS
- EIT ENGINEER IN TRAINING – A GRADUATE ENGINEER WHO HAS PASSED THE FUNDAMENTALS OF ENGINEERING EXAM
- ETT EXPERIENCED TESTING TECHNICIAN – AN EXPERIENCED TESTING TECHNICIAN WITH A MINIMUM OF 5 YEARS EXPERIENCE WITH THE STIPULATED TEST OR INSPECTION

AMERICAN CONCRETE INSTITUTE (ACI) CERTIFICATION

- ACI-CFTT CONCRETE FIELD TESTING TECHNICIAN – GRADE 1
- ACI-CCI CONCRETE CONSTRUCTION INSPECTOR
- ACI-LTT LABORATORY TESTING TECHNICIAN – GRADE 1&2
- ACI-STT STRENGTH TESTING TECHNICIAN

AMERICAN WELDING SOCIETY (AWS) CERTIFICATION

- AWS-CWI CERTIFIED WELDING INSPECTOR
- AWS/AISC-SSICERTIFIED STRUCTURAL STEEL INSPECTOR

PERIODIC, CONTINUOUS, OBSERVE, PERFORM, AND DOCUMENT ARE DEFINED AS IN THE SPECIFIC REFERENCE STANDARD.

SPECIAL CASES

1. PER IBC 1705.1.1, SPECIAL INSPECTIONS AND TESTS SHALL BE REQUIRED FOR PROPOSED WORK THAT IS, IN THE OPINION OF THE BUILDING OFFICIAL, UNUSUAL IN ITS NATURE SUCH AS:
   - A. MATERIALS AND SYSTEMS REQUIRED TO BE INSTALLED IN ACCORDANCE WITH ADDITIONAL MANUFACTURER'S INSTRUCTIONS THAT PRESCRIBE REQUIREMENTS NOT CONTAINED IN IBC OR IN STANDARDS REFERENCED BY IBC.
   - B. UNUSUAL DESIGN APPLICATIONS OF MATERIALS DESCRIBED IN IBC.
   - C. CONSTRUCTION MATERIALS AND SYSTEMS THAT ARE ALTERNATIVES TO MATERIALS AND SYSTEMS PRESCRIBED BY THIS CODE.

STRUCTURAL STEEL TESTING

1. PER IBC 1705.2, SPECIAL INSPECTIONS AND TESTS OF STEEL CONSTRUCTION SHALL BE PERFORMED.
2. CERTIFY WELDERS FOR THE WELD TYPES IN THE PROJECT AND CONDUCT INSPECTIONS AND TESTS AS REQUIRED, AS A MINIMUM, WELDERS SHALL BE AISC CERTIFIED. RECORD TYPES AND LOCATIONS OF DEFECTS FOUND IN WORK. RECORD WORK REQUIRED AND PERFORMED TO CORRECT DEFICIENCIES.
3. VISUALLY INSPECT 100% OF ALL FILLET WELDS.
4. ALL WELDS THAT FAIL SHALL BE REWELDED AND RETESTED UNTIL THEY PASS THE TEST. TEST TWO ADDITIONAL WELDS AT THE CONTRACTOR'S EXPENSE FOR EVERY WELD FAILURE.
5. BOLTS SHALL BE VISUALLY INSPECTED WHEN TWIST-OFF SPLINES ARE USED, OTHERWISE BOLTS SHALL BE SNUG TIGHT.
6. WHERE A COLD-FORMED STEEL TRUSS CLEAR SPAN IS 60 FEET OR GREATER, THE SPECIAL INSPECTOR SHALL VERIFY THAT THE TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING ARE INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE.
7. AFTER INSPECTIONS OF OPEN-WEB STEEL JOISTS AND JOIST GIRDERS ARE PERFORMED, THE REPORTS AND CERTIFICATES SHALL BE SUBMITTED BY THE OWNER OR OWNER'S AUTHORIZED AGENT TO THE BUILDING OFFICIAL.

CONCRETE TESTING

1. PER IBC 1705.3, SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION SHALL BE PERFORMED.
2. CONCRETE MIX DESIGNS SHALL BE SUBMITTED FOR REVIEW INDICATING CONFORMANCE WITH ACI 318, LATEST EDITION.
3. SLUMP TESTS, CONFORMING TO ASTM C143, SHALL BE TAKEN AT THE POINT OF DISCHARGE AT THE SAME RATE AS NOTED BELOW IN NOTE NUMBER 6.
4. AIR CONTENT TESTS CONFORMING TO ASTM C173, VOLUMETRIC METHOD FOR LIGHTWEIGHT OR NORMAL WEIGHT CONCRETE; ASTM C231 PRESSURE METHOD FOR NORMAL WEIGHT CONCRETE; SHALL BE TAKEN FOR EACH DAY'S POUR OF EACH TYPE OF AIR-ENTRAINED CONCRETE.
5. CONCRETE TEMPERATURE SHALL BE TESTED HOURLY WHEN AIR TEMPERATURE IS 40 DEG F (4 DEG C) AND BELOW, WHEN 80 DEG F (27 DEG C) AND ABOVE, AND EACH TIME A SET OF COMPRESSION TEST SPECIMENS IS MADE.
6. ONE SET OF FOUR COMPRESSION TEST SPECIMENS CONFORMING TO ASTM C31 SHALL BE MOLDED AND STORED FOR LABORATORY-CURED SPECIMENS. COMPRESSIVE STRENGTH TESTS SHALL CONFORM TO ASTM C39 AND SHALL CONSIST OF ONE SET FOR EACH DAY'S POUR EXCEEDING 5 CU. YDS. PLUS ADDITIONAL SETS FOR EACH 50 CU. YDS. MORE THAN THE FIRST 25 CU. YDS OF EACH CONCRETE CLASS PLACED IN ANY ONE DAY. ONE SPECIMEN SHALL BE TESTED AT 7 DAYS, TWO SPECIMENS SHALL BE TESTED AT 28 DAYS, AND ONE SPECIMEN SHALL BE RETAINED FOR LATER TESTING AS REQUIRED.
7. IN THE ABSENCE OF SUFFICIENT DATA OR DOCUMENTATION PROVIDING EVIDENCE OF CONFORMANCE TO QUALITY STANDARDS FOR MATERIALS IN THE LATEST EDITION OF ACI 318, THE BUILDING OFFICIAL SHALL REQUIRE TESTING OF MATERIALS IN ACCORDANCE WITH THE APPROPRIATE STANDARDS AND CRITERIA FOR THE MATERIAL IN THE LATEST EDITION OF ACI 318.
8. VERIFY CONCRETE IS BEING CONSOLIDATED IN ACCORDANCE WITH THE RECOMMENDATIONS OF ACI 318 AND ACI 309R, LATEST EDITION.
9. VERIFY THAT POST INSTALLED ANCHORS ARE AS SPECIFIED AND THAT ANCHORS ARE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS AND REQUIREMENTS.
10. PER IBC 1705.5.4, REPORTS OF PRECONSTRUCTION TESTS FOR SHOTCRETE IN ACCORDANCE WITH IBC SECTION 1908.5, SHALL BE SUBMITTED BY THE OWNER OR THE OWNER'S AUTHORIZED AGENT TO THE BUILDING OFFICIAL.
11. SPECIAL INSPECTIONS AND TESTS SHALL NOT BE REQUIRED FOR:
    - A. ISOLATED SPREAD CONCRETE FOOTINGS OF BUILDINGS THREE STORIES OR LESS ABOVE GRADE PLANE AND THAT ARE FULLY SUPPORTED ON EARTH OR ROCK.
    - B. CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS OF BUILDINGS THREES STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK WHERE:
      - a. THE FOOTINGS SUPPORT WALLS OF LIGHT-FRAME CONSTRUCTION
      - b. THE FOOTINGS ARE DESIGNED IN ACCORDANCE WITH IBC TABLE 1809.7.
      - c. THE STRUCTURAL DESIGN OF THE FOOTING IS BASED ON THE SPECIFIED COMPRESSIVE STRENGTH f'c, NOT MORE THAN 2,500 POUNDS PER SQUARE INCH (PSI), REGARDLESS OF THE COMPRESSIVE STRENGTH SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS OR USED IN THE FOOTING CONSTRUCTION.
    - C. NONSTRUCTURAL CONCRETE SLABS SUPPORTED DIRECTLY ON THE GROUND, INCLUDING PRESTRESSED SLABS ON GRADE, WHERE THE EFFECTIVE PRESTRESS IN THE CONCRETE IS LESS THAN 150 PSI.
    - D. CONCRETE FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH IBC TABLE 1807.1.6.2.
    - E. CONCRETE PATIOS, DRIVEWAYS, AND SIDEWALKS, ON GRADE.

SOIL SPECIAL INSPECTIONS

1. PER IBC 1705.6, SPECIAL INSPECTIONS AND TESTS OF EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT, AND LOAD-BEARING REQUIREMENTS SHALL BE PERFORMED.
2. DURING FILL PLACEMENT, VERIFY THAT PROPER MATERIALS AND PROCEDURES ARE USED IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT.
3. WHERE IBC SECTION 1803 DOES NOT REQUIRE REPORTING OF MATERIALS AND PROCEDURES FOR FILL PLACEMENT, VERIFY THAT THE IN-PLACE DRY DENSITY OF THE COMPACTED FILL IS NOT LESS THAN 90 PERCENT OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT DETERMINED IN ACCORDANCE WITH ASTM D1557.

REINFORCING STEEL INSTALLATION

1. DURING THE CAST-IN-PLACE CONCRETE STRUCTURAL MEMBER REINFORCING PLACEMENT OPERATIONS KEEP A COMPETENT TRAINED TECHNICIAN ASSIGNED TO THE PROJECT. INSPECT REINFORCING UTILIZING ACI 311.4R "GUIDE FOR CONCRETE INSPECTION" AS A GUIDE. SERVICES PROVIDED SHALL INCLUDE:
   - a. VERIFY TYPE AND GRADE OF ALL REINFORCING STEEL.
   - b. VERIFY REBAR IS FREE OF OIL, DIRT, EXCESSIVE RUST AND FROM DAMAGE IN SHIPMENT TO SITE.
   - c. VERIFY REINFORCING IS ADEQUATELY TIED, CHAIRED AND SUPPORTED TO PREVENT DISPLACEMENT DURING CONCRETE PLACEMENT. VERIFY MINIMUM AND MAXIMUM CLEAR DISTANCES BETWEEN BARS AND MINIMUM STRUCTURAL DISTANCE TO OUTSIDE OF CONCRETE. VERIFY QUANTITY, SIZE AND LOCATION OF REINFORCEMENT. VERIFY MINIMUM CONCRETE COVER IS MAINTAINED BETWEEN REBAR AND SURFACE OF CONCRETE.
   - d. VERIFY SIZE AND PLACEMENT OF REBAR. VERIFY LAP LENGTHS, LOCATIONS AND STAGGERS AND VERIFY BENDS FOR MINIMUM DIAMETER, SLOPE AND LENGTH. VERIFY HOOKED BAR LENGTHS AND LOCATIONS.

EARTHWORK TESTING

1. DURING EARTHWORK OPERATIONS KEEP A COMPETENT TRAINED TECHNICIAN ASSIGNED TO THE PROJECT. SERVICES PROVIDED SHALL INCLUDE:
   - a. OBSERVE STRIPPING OPERATIONS AND EVALUATE THE REQUIRED STRIPPING DEPTH DURING THESE OPERATIONS.
   - b. OBSERVE PROOFROLLING OPERATIONS AFTER SITE STRIPPING. DETERMINE IF ANY SOFT SPOTS NEED TO BE UNDERCUT TO FIRM SOILS, REPLACED WITH SELECT FILL AND RECOMPACTED.
   - c. VERIFY THAT THE SUBGRADE SHALL THEN BE SCARIFIED AND MOISTURE CONDITIONED TO AN EIGHT (8) INCH DEPTH AND THEN RECOMPACTED TO BETWEEN 95 AND 100 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR DENSITY TEST (ASTM D698). THE MOISTURE CONTENT SHALL BE BETWEEN OPTIMUM AND +3 PERCENT OF THE OPTIMUM MOISTURE CONTENT. PROVIDE A MINIMUM OF FOUR (4) FIELD DENSITY TESTS ON THE SUBGRADE OR ONE (1) FOR EVERY 2,500 SQUARE FEET WHICHEVER IS GREATER.
   - d. STRUCTURAL SELECT FILL PAD MATERIAL SHALL BE TESTED FOR ACCEPTABILITY AND A MOISTURE DENSITY CURVE SHALL BE ESTABLISHED. SELECT FILL MATERIAL SHALL BE AN INORGANIC SANDY CLAY WITH LIQUID LIMIT OF 26 AND PLASTICITY INDEX BETWEEN 10 AND 20.
   - e. SELECT FILL SHALL BE PLACED IN SIX (6) INCH LOOSE LIFTS AND COMPACTED TO BETWEEN 95 AND 100 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR DENSITY TEXT (ASTM D698). THE MOISTURE CONTENT SHALL BE BETWEEN OPTIMUM AND +3 PERCENT OF THE OPTIMUM MOISTURE CONTENT FOR SELECT FILL. VERIFY THAT SELECT FILL MATERIAL EXTENDS TO 5'-0" BEYOND THE BUILDING PERIMETER.
   - f. SELECT FILL MATERIAL SHALL BE TESTED DURING PLACEMENT OF EACH LIFT FOR THE ATTERBERG LIMITS IN ACCORDANCE WITH ASTM D4318-98 METHOD B "STANDARD TEST METHOD FOR LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS" TO VERIFY THAT THE SELECT FILL MATERIAL IS IN ACCORDANCE WITH THE ORIGINALLY APPROVED SELECT FILL MATERIAL. PROVIDE A MINIMUM OF ONE (1) TEST PER LIFT OR ONE (1) FOR EVERY 2,500 SQUARE FEET WHICHEVER IS GREATER WITH A MAXIMUM OF TEN (10) PER LIFT.
   - g. OBSERVE THE EXCAVATION DAILY AND ENSURE THAT THE CONTRACTOR MAINTAINS A CLEAN EXCAVATION THAT IS FREE OF WATER 100% OF THE TIME. CONTRACTOR SHALL PROVIDE PUMPS AS REQUIRED TO REMOVE ANY WATER AT ALL TIMES.
   - h. OBSERVE GRADING OPERATIONS TO ENSURE THAT PROPER DRAINAGE AWAY FROM THE BUILDING PAD IS PROVIDED.

SPECIAL INSPECTION OF FABRICATED ITEMS

PER IBC 1704.2.5, WHERE FABRICATION OF STRUCTURAL, LOAD-BEARING OR LATERAL LOAD-RESISTING MEMBERS OR ASSEMBLIES IS BEING CONDUCTED ON THE PREMISES OF A FABRICATOR'S SHOP:

EITHER:

- A. SPECIAL INSPECTIONS OF THE FABRICATED ITEMS SHALL BE PERFORMED DURING FABRICATION AT THE FABRICATOR'S SHOP, OR;
- B. THE FABRICATOR SHALL HAVE BEEN APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTIONS IN ACCORDANCE WITH IBC 1704.2.5.1. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE OWNER OR THE OWNER'S AUTHORIZED AGENT FOR SUBMITTAL TO THE BUILDING OFFICIAL AS SPECIFIED IN IBC SECTION 1704.5 STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

TESTING FOR SEISMIC RESISTANCE - IBC 2021

1. PER IBC 1705.13, TESTING FOR SEISMIC RESISTANCE ARE REQUIRED AS NOTED BELOW:
2. NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL IN THE SEISMIC FORCE-RESISTING SYSTEMS OF BUILDINGS AND STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **B, C, D, E OR F** SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341. EXCEPTION: NONDESTRUCTIVE TESTING IS NOT REQUIRED IN THE SEISMIC FORCE-RESISTING SYSTEMS OF BUILDINGS AND STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **B OR C** THAT ARE NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE, WITH A RESPONSE MODIFICATION COEFFICIENT, ***R*, OF 3 OR LESS**, EXCLUDING CANTILEVER COLUMN SYSTEMS.
3. NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN THE SEISMIC FORCE-RESISTING SYSTEMS OF BUILDINGS AND STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **B, C, D, E OR F** OTHER THAN THOSE COVERED IN SECTION 1705.13.L.1, INCLUDING STRUTS, COLLECTORS, CHORDS AND FOUNDATION ELEMENTS, SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341. EXCEPTION: NONDESTRUCTIVE TESTING OF *STRUCTURAL STEEL ELEMENTS* IS NOT REQUIRED IN THE SEISMIC FORCE-RESISTING SYSTEMS OF BUILDINGS AND STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **B OR C** WITH A RESPONSE MODIFICATION COEFFICIENT, ***R*, OF 3 OR LESS**.
4. FOR STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **C, D, E OR F** AND WITH *DESIGNATED SEISMIC SYSTEMS* THAT ARE SUBJECT TO THE REQUIREMENTS OF SECTION 13.2.2 OF ASCE 7 FOR CERTIFICATION, THE *REGISTERED DESIGN PROFESSIONAL* SHALL SPECIFY ON THE *APPROVED CONSTRUCTION DOCUMENTS* THE REQUIREMENTS TO BE MET BY ANALYSIS, TESTING OR EXPERIENCE DATA AS SPECIFIED THEREIN. *CERTIFICATES OF COMPLIANCE* DOCUMENTING THAT THE REQUIREMENTS ARE MET SHALL BE SUBMITTED TO THE *BUILDING OFFICIAL* AS SPECIFIED IN SECTION 1704.5.
5. SEISMIC ISOLATION SYSTEMS IN SEISMICALLY ISOLATED STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **B, C, D, E OR F** SHALL BE TESTED IN ACCORDANCE WITH SECTION 17.8 OF ASCE 7.

SEISMIC SPECIAL INSPECTIONS - IBC 2021

PER IBC 1705.12, INSPECTIONS FOR SEISMIC RESISTANCE ARE REQUIRED AS NOTED BELOW:

1. *SPECIAL INSPECTIONS* OF STRUCTURAL STEEL IN THE SEISMIC FORCE-RESISTING SYSTEMS OF BUILDINGS AND STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **B, C, D, E OR F** SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341.
2. *SPECIAL INSPECTIONS* OF *STRUCTURAL STEEL ELEMENTS* IN THE SEISMIC FORCE-RESISTING SYSTEMS OF BUILDINGS AND STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **B, C, D, E OR F** OTHER THAN THOSE COVERED ABOVE, INCLUDING STRUTS, COLLECTORS, CHORDS AND FOUNDATION ELEMENTS, SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE REQUIREMENTS OF AISC 341.
3. COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION. FOR THE SEISMIC FORCE-RESISTING SYSTEMS OF STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **C, D, E OR F**, PERIODIC SPECIAL INSPECTION SHALL BE REQUIRED.
4. STRUCTURAL WOOD CONSTRUCTION. FOR THE SEISMIC FORCE-RESISTING SYSTEMS OF STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **C, D, E OR F**, PERIODIC SPECIAL INSPECTION SHALL BE REQUIRED.
5. DESIGNATED SEISMIC SYSTEMS. FOR STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **C, D, E OR F**, THE SPECIAL INSPECTOR SHALL EXAMINE *DESIGNATED SEISMIC SYSTEMS* REQUIRING SEISMIC QUALIFICATION IN ACCORDANCE WITH SECTION 13.2.2 OF ASCE 7 AND VERIFY THAT THE LABEL, ANCHORAGE AND MOUNTING CONFORM TO THE *CERTIFICATE OF COMPLIANCE.*
6. STORAGE RACKS. *PERIODIC SPECIAL INSPECTION* IS REQUIRED FOR THE ANCHORAGE OF STORAGE RACKS THAT ARE 8 FEET OR GREATER IN HEIGHT IN STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **D, E OR F**.
7. SEISMIC ISOLATION SYSTEMS. *PERIODIC SPECIAL INSPECTION* SHALL BE PROVIDED FOR SEISMIC ISOLATION SYSTEMS IN SEISMICALLY ISOLATED STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **B, C, D, E OR F** DURING THE FABRICATION AND INSTALLATION OF ISOLATOR UNITS AND ENERGY DISSIPATION DEVICES.
8. COLD-FORMED STEEL SPECIAL BOLTED MOMENT FRAMES. *PERIODIC SPECIAL INSPECTION* SHALL BE PROVIDED FOR THE INSTALLATION OF COLD-FORMED STEEL SPECIAL BOLTED MOMENT FRAMES IN THE *SEISMIC FORCE-RESISTING SYSTEMS* OF STRUCTURES ASSIGNED TO *SEISMIC DESIGN CATEGORY* **D, E OR F**.

EXCEPTION: SPECIAL INSPECTIONS ITEMIZED IN SECTIONS 1705.12.1 THROUGH 1705.12.9 ARE NOT REQUIRED FOR STRUCTURES DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH ONE OF THE FOLLOWING:

1. THE STRUCTURE CONSISTS OF LIGHT-FRAME CONSTRUCTION; THE DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, *SDS'* AS DETERMINED IN SECTION 1613.3.4, DOES NOT EXCEED 0.5; AND THE BUILDING HEIGHT OF THE STRUCTURE DOES NOT EXCEED 35 FEET.
2. THE SEISMIC FORCE-RESISTING SYSTEM OF THE STRUCTURE CONSISTS OF REINFORCED MASONRY OR REINFORCED CONCRETE; THE DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, *SDS'* AS DETERMINED IN SECTION 1613.3.4, DOES NOT EXCEED 0.5; AND THE BUILDING HEIGHT OF THE STRUCTURE DOES NOT EXCEED 25 FEET.

MASONRY TESTING

1. PER IBC 1705.4, SPECIAL INSPECTIONS AND TESTS SHALL BE REQUIRED FOR MASONRY CONSTRUCTION THAT SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE PROGRAM REQUIREMENTS OF TMS 402 AND TMS 602.
2. SPECIAL INSPECTIONS SHALL NOT BE REQUIRED FOR:
   - A. EMPIRICALLY DESIGNED MASONRY, GLASS UNIT MASONRY, OR MASONRY VENEER DESIGNED IN ACCORDANCE WITH IBC SECTION 2109, 2110, OR CHAPTER 14, RESPECTIVELY, WHERE THEY ARE PART OF A STRUCTURE CLASSIFIED AS RISK CATEGORY I, II, OR III.
   - B. MASONRY FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH TABLE 1807.1.6.3(1), 1807.1.6.3(2), 1807.1.6.3(3), OR 1807.1.6.3(4).
   - C. MASONRY FIREPLACES, MASONRY HEATERS OR MASONRY CHIMNEYS INSTALLED OR CONSTRUCTED IN ACCORDANCE WITH SECTION 2111, 2112, OR 2113, RESPECTIVELY.
3. SPECIAL INSPECTIONS AND TESTS FOR EMPIRICALLY DESIGNED MASONRY, GLASS UNIT MASONRY OR MASONRY VENEER DESIGNED IN ACCORDANCE WITH SECTION 2109, 2110, OR CHAPTER 14, RESPECTIVELY, WHERE THEY ARE PART OF A STRUCTURE CLASSIFIED AS RISK CATEGORY IV SHALL BE PERFORMED IN ACCORDANCE WITH TMS 402, LEVEL B QUALITY ASSURANCE.
4. LEVEL A QUALITY ASSURANCE SHALL BE THE MINIMUM REQUIREMENT FOR MASONRY IN RISK CATEGORY I, II, OR III STRUCTURES AND DESIGNED IN ACCORDANCE WITH TMS PART 4 (PRESCRIPTIVE DESIGN METHODS) OR TMS APPENDIX A (EMPIRICAL DESIGN OF MASONRY).
5. LEVEL B QUALITY ASSURANCE SHALL BE THE MINIMUM REQUIREMENT FOR MASONRY IN RISK CATEGORY IV STRUCTURES AND DESIGNED IN ACCORDANCE WITH TMS CHAPTER 12 OR 13; OR IT SHALL BE THE MINIMUM REQUIREMENT FOR MASONRY IN RISK CATEGORY I, II, OR III STRUCTURES AND DESIGNED IN ACCORDANCE WITH CHAPTERS OTHER THAN TMS PART 4 (PRESCRIPTIVE DESIGN METHODS) OR TMS APPENDIX A (EMPIRICAL DESIGN OF MASONRY).
6. LEVEL C QUALITY ASSURANCE SHALL BE THE MINIMUM REQUIREMENT FOR MASONRY IN RISK CATEGORY IV STRUCTURES AND DESIGNED IN ACCORDANCE WITH CHAPTERS OTHER THAN TMS PART 4 (PRESCRIPTIVE DESIGN METHODS) OR TMS APPENDIX A (EMPIRICAL DESIGN OF MASONRY).

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION / INSPECTION	SOIL / FOUNDATION INSPECTION		
IBC SECTION 1705.6, TABLE 1705.6	EXTENT CONTINUOUS, PERIODIC	COMMENTS	AGENT PE/GE, EIT, OR ETT
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC	IBC 1705.6, TABLE 1705.6	ETT
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC	IBC 1705.6, TABLE 1705.6	ETT
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	PERIODIC	IBC 1705.6, TABLE 1705.6	ETT
VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	CONTINUOUS	IBC 1705.6, TABLE 1705.6	ETT
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC	IBC 1705.6, TABLE 1705.6	ETT
THE APPROVED GEOTECHNICAL REPORT AND THE CONSTRUCTION DOCUMENTS PREPARED BY THE REGISTERED DESIGN PROFESSIONALS SHALL BE USED TO DETERMINE COMPLIANCE.			

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION / INSPECTION	CONCRETE INSPECTION		
IBC 2021 SECTION 1705.6, TABLE 1705.6	EXTENT CONTINUOUS, PERIODIC	COMMENTS	AGENT PE/GE, EIT, OR ETT
INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	PERIODIC	ACI 318: Ch. 20, 25.2, 25.3, 26.6.1 - 26.6.3 IBC 1908.4	ETT
REINFORCING BAR WELDING: A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706; B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16", AND C. INSPECT ALL OTHER WELDS	 PERIODIC PERIODIC CONTINUOUS	AWS D1.4 ACI 318: 26.6.4	AWS-CWI
INSPECTION OF ANCHORS CAST IN CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED.	PERIODIC	ACI 318: 17.8.2	ETT
INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS: A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	CONTINUOUS	ACI 318: 17.8.2.4	ETT
B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED ABOVE.	PERIODIC	ACI 318: 17.8.2	ETT
VERIFYING USE OF REQUIRED MIX DESIGN.	PERIODIC	ACI 318: Ch. 19, 26.4.3, 26.4.4 IBC 1904.1, 1904.2, 1908.2, 1908.3	ETT
PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	CONTINUOUS	ASTM C 172 ASTM C 31 ACI 318: 26.5, 26.12 IBC 1908.10	ACI-CFTT OR ACI-STT
INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS	ACI 318: 26.5 IBC 1908.6, 1908.7, 1908.8	ETT
INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	ACI 318: 26.5.3-26.5.5 IBC 1908.9	ETT
VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	PERIODIC	ACI 318: 26.11.2	ETT
INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	PERIODIC	ACI 318: 26.11.1.2(b)	ETT

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12/21/22

Drawing Title
STRUCTURAL SPECIAL INSPECTIONS

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date **12/21/2022** | Checked SBJ | Drawn ZAF

Project Number **589-704**

Building Number **26**

Drawing Number **SA010**

Drawing # 16 OF 190

Veterans Health Administration

VA U.S. Department of Veterans Affairs

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VA FORM 08-6231

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION / INSPECTION	BOLTING INSPECTIONS		
IBC SECTION 1705.2, AISC 360, LATEST EDITION	EXTENT OBSERVE, PERFORM	COMMENTS	AGENT PE/GE, EIT, OR ETT
INSPECTIONS PRIOR TO BOLTING	-	-	-
MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	PERFORM	TABLE N5.6-1	ETT
FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	OBSERVE	TABLE N5.6-1	ETT
CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	OBSERVE	TABLE N5.6-1	ETT
CORRECT BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	OBSERVE	TABLE N5.6-1	ETT
CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	OBSERVE	TABLE N5.6-1	ETT
PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	OBSERVE	TABLE N5.6-1	ETT
PROTECTED STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	OBSERVE	TABLE N5.6-1	ETT
INSPECTIONS DURING BOLTING	-	-	-
FASTENER ASSEMBLIES PLACED IN ALL HOLES AND WASHERS AND NUTS ARE POSITIONED AS REQUIRED	OBSERVE	TABLE N5.6-2	ETT
JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	OBSERVE	TABLE N5.6-2	ETT
FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	OBSERVE	TABLE N5.6-2	ETT
FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	OBSERVE	TABLE N5.6-2	ETT
INSPECTIONS AFTER BOLTING	-	-	-
DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	PERFORM	TABLE N5.6-3	ETT

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION / INSPECTION	MASONRY LEVEL C INSPECTION				
IBC SECTION 1705.4, TMS 402 & 602	EXTENT CONTINUOUS, PERIODIC	COMMENTS			AGENT PE/GE, EIT, OR ETT
		IBC SECTION	TMS 402 ACI 530 ASCE 5	TMS 602 ACI 530.1 ASCE 6	
VERIFY COMPLIANCE WITH APPROVED SUBMITTALS	PERIODIC			ART. 1.5	ETT
1. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:	-	-			-
A. PROPORTIONS OF SITE-PREPARED MORTAR, GROUT, AND PRESTRESSING GROUT FOR BONDED TENDONS	PERIODIC			ART. 2.1, 2.6A-C, 2.4 G.1.b	ETT
B. PLACEMENT OF MASONRY UNITS & CONSTRUCTION OF MORTAR JOINTS	PERIODIC			ART. 3.3B	ETT
C. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES	PERIODIC			ART. 2.4B, 2.4H	ETT
D. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES	PERIODIC			ART. 3.4, 3.6A	ETT
E. PROPERTIES OF THIN-BED MORTAR FOR ACC MASONRY	CONTINUOUS			ART. 2.1C	ETT
G. GROUT SPACE PRIOR TO GROUTING	PERIODIC			ART. 3.2D, 3.2F	ETT
H. SIZE AND LOCATION OF STRUCTURAL ELEMENTS	PERIODIC			ART. 3.3F	ETT
I. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION	PERIODIC		SEC. 1.2.1(e) 6.1.4.3, 6.2.1		ETT
J. WELDING OF REINFORCEMENT	CONTINUOUS		SEC. 8.1.6.7.2, 9.3.3.4(c), 11.3.3.4(b)		AWS-CWI
K. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40 DEG. F) OR HOT WEATHER (TEMPERATURE ABOVE 90 DEG. F)	PERIODIC			ART. 1.8C 1.8D	ETT
L. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	CONTINUOUS			ART. 3.6B	ETT
M. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	CONTINUOUS			ART. 3.5, 3.6C	ETT
N. PLACEMENT OF ACC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	CONTINUOUS			ART. 3.3 B.9, 3.3 F.1.b	ETT
OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	PERIODIC			ART. 1.4 B.2.a-c.3, 1.4 B.3-4	ETT

LEVEL C QUALITY ASSURANCE MINIMUM TESTS:

VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.5 B.1.b.3 FOR SELF-CONSOLIDATING GROUT.

VERIFICATION OF f'm AND f'ACC IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.4B PRIOR TO CONSTRUCTION AND FOR EVERY 5,000 SQ FT DURING CONSTRUCTION.

VERIFICATION OF PROPORTIONS OF MATERIALS IN PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT, AS DELIVERED TO THE PROJECT SITE.

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION / INSPECTION	STEEL DECK MECHANICAL FASTENING INSPECTIONS		
IBC 2021 1705.2.2, SDI QA/QC - LATEST EDITION	EXTENT OBSERVE, PERFORM	COMMENTS	AGENT PE/GE, EIT, OR ETT
INSPECTIONS PRIOR TO MECHANICAL FASTENING	-	-	-
MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS	OBSERVE	TABLE 1.6	ETT
PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION	OBSERVE	TABLE 1.6	ETT
PROPER STORAGE FOR MECHANICAL FASTENERS	OBSERVE	TABLE 1.6	ETT
INSPECTIONS DURING MECHANICAL FASTENING	-	-	-
FASTENERS ARE POSITIONED AS REQUIRED	OBSERVE	TABLE 1.7	ETT
FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS	OBSERVE	TABLE 1.7	ETT
INSPECTIONS AFTER MECHANICAL FASTENING	-	-	-
CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS	PERFORM	TABLE 1.8	ETT
CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS	PERFORM	TABLE 1.8	ETT
CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS	PERFORM	TABLE 1.8	ETT
VERIFY REPAIR ACTIVITIES	PERFORM	TABLE 1.8	ETT
DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS	PERFORM	TABLE 1.8	ETT

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION / INSPECTION	STEEL DECK WELDING INSPECTIONS		
IBC 2021 1705.2.2, SDI QA/QC -2011	EXTENT OBSERVE, PERFORM	COMMENTS	AGENT PE/GE, EIT, OR ETT
INSPECTIONS PRIOR TO WELDING	-	-	-
WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE	OBSERVE	TABLE 1.3	ETT
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	OBSERVE	TABLE 1.3	ETT
MATERIAL IDENTIFICATION (TYPE/GRADE)	OBSERVE	TABLE 1.3	ETT
CHECK WELDING EQUIPMENT	OBSERVE	TABLE 1.3	ETT
INSPECTIONS DURING WELDING	-	-	-
USE OF QUALIFIED WELDERS	OBSERVE	TABLE 1.4	ETT
CONTROL AND HANDLING OF WELDING CONSUMABLES	OBSERVE	TABLE 1.4	ETT
ENVIRONMENTAL CONDITIONS (WIND SPEED, MOISTURE, TEMPERATURE)	OBSERVE	TABLE 1.4	ETT
WPS FOLLOWED	OBSERVE	TABLE 1.4	ETT
INSPECTIONS AFTER WELDING	-	-	-
VERIFY SIZE AND LOCATION OF WELDS, INCLUDING SUPPORT, SIDELAP, AND PERIMETER WELDS	PERFORM	TABLE 1.5	ETT
WELDS MEET VISUAL ACCEPTANCE CRITERIA	PERFORM	TABLE 1.5	ETT
VERIFY REPAIR ACTIVITIES	PERFORM	TABLE 1.5	ETT
DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	PERFORM	TABLE 1.5	ETT

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION / INSPECTION	STEEL DECK PLACEMENT INSPECTIONS		
IBC 2021 1705.2.2, SDI QA/QC -2011	EXTENT OBSERVE, PERFORM	COMMENTS	AGENT PE/GE, EIT, OR ETT
INSPECTIONS PRIOR TO DECK PLACEMENT	-	-	-
VERIFY COMPLIANCE OF MATERIALS (DECK AND ALL DECK ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS	PERFORM	TABLE 1.1	ETT
DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES	PERFORM	TABLE 1.1	ETT
INSPECTIONS AFTER DECK PLACEMENT	-	-	-
VERIFY COMPLIANCE OF DECK AND ALL DECK ACCESSORIES INSTALLATION WITH CONSTRUCTION DOCUMENTS	PERFORM	TABLE 1.2	ETT
VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS	PERFORM	TABLE 1.2	ETT
DOCUMENT ACCEPTANCE OR REJECTION OF INSTALLATION OF DECK AND DECK ACCESSORIES	PERFORM	TABLE 1.2	ETT

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION / INSPECTION	INSPECTION PRIOR TO WELDING		
IBC 2021 SECTION 1705.2, AISC 360-16	EXTENT OBSERVE, PERFORM	COMMENTS	AGENT PE/GE, EIT, OR ETT
WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	OBSERVE	TABLE N5.4-1	ETT
WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	PERFORM	TABLE N5.4-1	ETT
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	PERFORM	TABLE N5.4-1	ETT
MATERIAL IDENTIFICATION (TYPE/GRADE)	OBSERVE	TABLE N5.4-1	ETT
WELDER IDENTIFICATION SYSTEM	OBSERVE	TABLE N5.4-1	ETT
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY) • JOINT PREPARATION • DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION) • BACKING TYPE AND FIT (IF APPLICABLE)	OBSERVE	TABLE N5.4-1	ETT
FIT-UP OF CJP GROOVE WELDS OF HSS T-, Y, AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY) • JOINT PREPARATION • DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION) • BACKING TYPE AND FIT (IF APPLICABLE)	OBSERVE	TABLE N5.4-1	ETT
CONFIGURATION AND FINISH OF ACCESS HOLES	OBSERVE	TABLE N5.4-1	ETT
FIT-UP OF FILLET WELDS • DIMENSIONS (ALIGNMENT, GAPS AT ROOT) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION)	OBSERVE	TABLE N5.4-1	ETT
CHECK WELDING EQUIPMENT	OBSERVE	TABLE N5.4-1	ETT

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION / INSPECTION	INSPECTION DURING WELDING		
IBC 2021 SECTION 1705.2, AISC 360-16	EXTENT OBSERVE, PERFORM	COMMENTS	AGENT PE/GE, EIT, OR ETT
CONTROL AND HANDLING OF WELDING CONSUMABLES • PACKAGING • EXPOSURE CONTROL	OBSERVE	TABLE N5.4-2	ETT
NO WELDING OVER CRACKED TACK WELDS	OBSERVE	TABLE N5.4-2	ETT
ENVIRONMENTAL CONDITIONS • WIND SPEED WITHIN LIMITS • PRECIPITATION AND TEMPERATURE	OBSERVE	TABLE N5.4-2	ETT
WPS FOLLOWED • SETTINGS ON WELDING EQUIPMENT • TRAVEL SPEED • SELECTED WELDING MATERIALS • SHIELDING GAS TYPE/FLOW RATE • PREHEAT APPLIED • INTERPASS TEMPERATURE MAINTAINED (MIN/MAX) • PROPER POSITION (F,V,H,OH)	OBSERVE	TABLE N5.4-2	ETT
WELDING TECHNIQUES • INTERPASS AND FINAL CLEANING • EACH PASS WITHIN PROFILE LIMITATIONS • EACH PASS MEETS QUALITY REQUIREMENTS	OBSERVE	TABLE N5.4-2	ETT
PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	PERFORM	TABLE N5.4-2	ETT

SCHEDULE OF SPECIAL INSPECTIONS

VERIFICATION / INSPECTION	INSPECTION AFTER WELDING		
IBC 2021 SECTION 1705.2, AISC 360-16	EXTENT OBSERVE, PERFORM	COMMENTS	AGENT PE/GE, EIT, OR ETT
WELDS CLEANED	OBSERVE	TABLE N5.4-3	ETT
SIZE, LENGTH, AND LOCATION OF WELDS	PERFORM	TABLE N5.4-3	ETT
WELDS MEET VISUAL ACCEPTANCE CRITERIA • CRACK PROHIBITION • WELD / BASE-METAL FUSION • CRATER CROSS SECTION • WELD PROFILES • WELD SIZE • UNDERCUT • POROSITY	PERFORM	TABLE N5.4-3	ETT
ARC STRIKES	PERFORM	TABLE N5.4-3	ETT
K - AREA	PERFORM	TABLE N5.4-3	ETT
WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES	PERFORM	TABLE N5.4-3	ETT
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	PERFORM	TABLE N5.4-3	ETT
REPAIR ACTIVITIES	PERFORM	TABLE N5.4-3	ETT
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	PERFORM	TABLE N5.4-3	ETT
NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR	OBSERVE	TABLE N5.4-3	ETT

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KS ENGR REG. NO. E-2586, EXP. 12/31/2021

STAMP:

SARAH J. BRISCOE LICENSED 28649 KANSAS PROFESSIONAL ENGINEER 12/21/22

Drawing Title
STRUCTURAL SPECIAL INSPECTIONS

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date **12/21/2022** | Checked SJB | Drawn ZAF

Project Number **589-704**

Building Number **26**

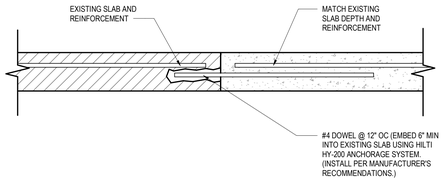
Drawing Number **SA011**

Drawing # 17 OF 190

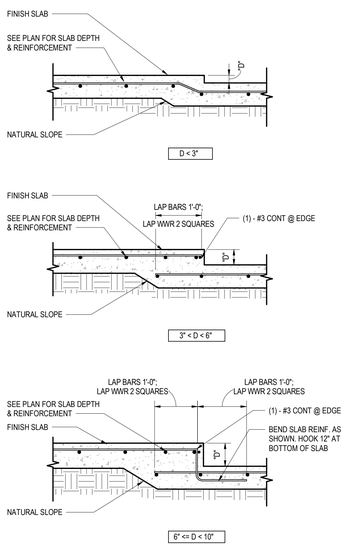
Veterans Health Administration

VA | U.S. Department of Veterans Affairs

VA FORM 08-6231

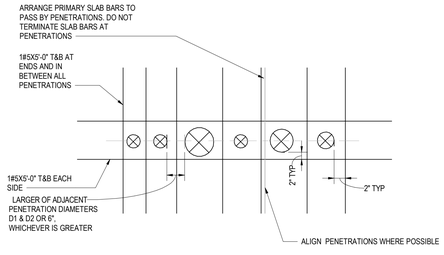


11 SLAB REPAIR DETAIL
NTS



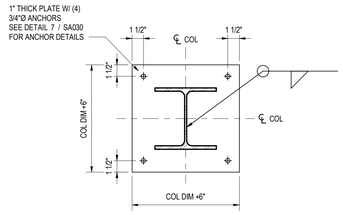
NOTES:
1. COORDINATE DEPTH AND LOCATION OF ALL FLOOR DEPRESSIONS WITH ARCHITECTURAL DRAWINGS.
2. PROVIDE (1) - #4 X 4'-0" TOP AT INTERIOR CORNERS OF ALL DEPRESSIONS.

10 TYPICAL SLAB RECESS DETAIL
NTS

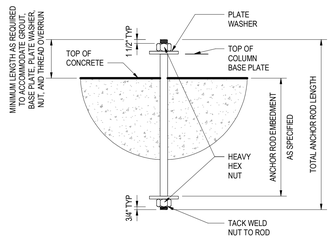


NOTE:
DETAIL DOES NOT APPLY FOR PENETRATIONS LARGER THAN 12" IN DIAMETER. SEE TYP SLAB OPENING DETAIL FOR SUCH PENETRATIONS.

9 TYPICAL ALIGNED CIP SLAB PENETRATIONS
NTS



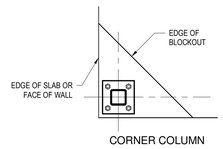
8 TYPICAL COLUMN BASE PLATE DETAIL
NTS



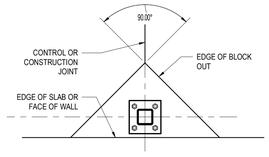
NOTES:

1. ANCHOR BOLT SHALL BE F1554 GRADE 36 WITH HEAVY HEX NUT. EMBEDMENT LENGTHS CORRESPONDING TO ANCHOR BOLT DIAMETERS SHALL BE:
   - A. 5/8" DIAMETER BOLT: MIN 10" EMBEDMENT
   - B. 3/4" DIAMETER BOLT: MIN 12" EMBEDMENT
   - C. 1" DIAMETER BOLT: MIN 16" EMBEDMENT
   - D. 1-1/4" DIAMETER BOLT: MIN 20" EMBEDMENT
   - E. 1-1/2" DIAMETER BOLT: MIN 24" EMBEDMENT
2. UNLESS NOTED OTHERWISE, ALL ANCHOR RODS NUTS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITIONED AS DEFINED BY AISC AFTER THE CONCRETE IS AT LEAST 14 DAYS OLD.
3. THE HOLE IN THE PLATE WASHER SHALL BE 1/16" LARGER THAN THE DIAMETER OF THE ANCHOR BOLT.

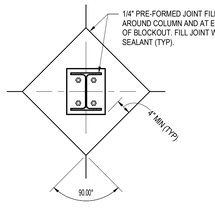
7 TYPICAL ANCHOR ROD DETAIL
NTS



CORNER COLUMN



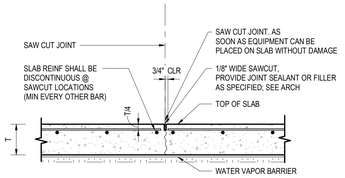
EXTERIOR COLUMN



INTERIOR COLUMN

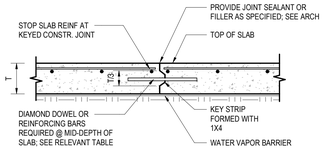
NOTE:
PROVIDE SONOTUBE BLOCKOUT WHEN CONTROL / CONSTRUCTION JOINTS DO NOT EXTEND FROM ALL CORNERS OF BLOCKOUT

6 SOG JOINTS AT COLUMNS
NTS



JOINT SPACING TABLE	
SLAB DEPTH (T)(in)	MAX DISTANCE BETWEEN JOINTS (FT)
5	13
6	15
7	18
8	20
9	23
10	25

CONTRACTION JOINT



REINFORCING DOWEL TABLE			
SLAB DEPTH (T)(in)	DOWEL SIZE	DOWEL LENGTH(in)	DOWEL SPACING(in)
5	#6	16	12
6	#6	16	12
7	#8	18	12
8	#8	18	12
9	#10	18	12
10	#10	18	12

DIAMOND DOWEL TABLE		
SLAB DEPTH (T)(in)	PLATE SIZE	DOWEL SPACING(in)
5 - 6	1/4"	18
7 - 9	3/8"	18
>9	3/4"	20

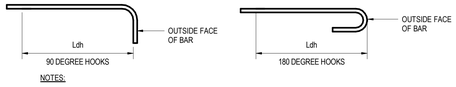
CONSTRUCTION JOINT

5 CONTRACTION/CONSTRUCTION JOINT DETAIL
NTS

DEVELOPMENT LENGTHS OF STANDARD HOOKS IN TENSION

GRADE 60 REINFORCEMENT, NORMAL CONCRETE

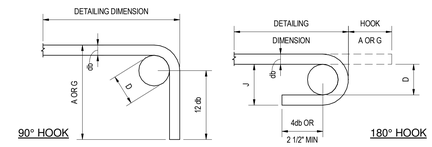
BAR SIZE	fc=3000PSI		fc=4000PSI		fc=5000PSI		fc=6000PSI		fc=7000PSI		fc=8000PSI		BAR SIZE
	Lhb	0.7 Lhb	Lhb	0.7 Lhb	Lhb	0.7 Lhb	Lhb	0.7 Lhb	Lhb	0.7 Lhb	Lhb	0.7 Lhb	
#3	9	7	8	6	7	6	6	6	6	6	6	6	#3
#4	11	8	10	7	9	7	8	6	8	6	7	6	#4
#5	14	10	12	9	11	8	10	7	9	7	9	7	#5
#6	17	12	15	11	13	10	12	9	11	8	11	8	#6
#7	20	14	17	12	15	11	14	10	13	10	12	9	#7
#8	22	16	19	14	17	12	16	12	15	11	14	10	#8
#9	25	18	22	16	20	14	18	13	17	12	16	12	#9
#10	28	20	25	18	22	16	20	14	19	14	18	13	#10
#11	31	22	27	19	24	17	22	16	21	15	19	14	#11
#14	38	--	33	--	29	--	27	--	25	--	23	--	#14
#18	50	--	43	--	39	--	35	--	33	--	31	--	#18



NOTES:

1. Ldh = DEVELOPMENTAL LENGTH OF STANDARD HOOKS IN TENSION (INCHES).
2. Ldh = Lhb UNLESS CONDITIONS OF 3 SATISFIED.
3. Ldh = 0.7 Lhb FOR #11 BARS AND SMALLER WHEN SIDE COVER (NORMAL TO PLANE OF HOOK) IS NOT LESS THAN 2 1/2 INCHES AND FOR 90 DEGREE HOOKS COVER ON BAR EXTENSION BEYOND HOOK IS NOT LESS THAN 2 INCHES.
4. HOOKS ARE NOT CONSIDERED EFFECTIVE FOR DEVELOPING BAR IN COMPRESSION.
5. Ldh SHALL BE MULTIPLIED BY 1.2 FOR EPOXY-COATED HOOKED REINFORCEMENT BARS.

4 SPLICE & DEVELOPMENT SCHEDULE FOR HOOKS
NTS



END HOOK DIMENSIONS				
BAR SIZE	FINISHED BEND DIAMETER D, (INCHES)	180 ° HOOKS		90° HOOK
		A OR G' (INCHES)	J, (INCHES)	A OR G' (INCHES)
#3	2 ¼	5	3	6
#4	3	6	4	8
#5	3 ¾	7	5	10
#6	4 ½	8	6	12
#7	5 ¼	10	7	14
#8	6	11	8	16
#9	9 ½	15	11 ¾	19
#10	10 ¼	17	13 ¼	22
#11	12	19	14 ¾	24
#14	18 ¼	27	21 ¼	31
#18	24	36	28 ½	41

D= INSIDE DIAMETER OF BEND

3 TYPICAL END HOOK TYPES
NTS

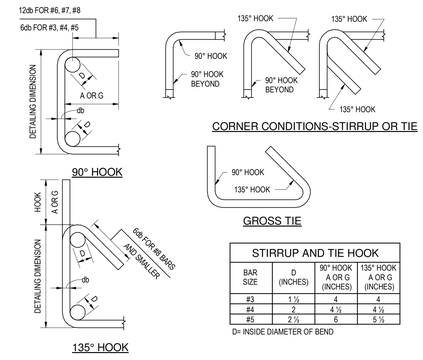
DEVELOPMENT AND LAP SPLICE SCHEDULE													
	f'c = 3500 psi							f'c = 4000 psi					
	EMBEDMENT			LAP SPLICE				EMBEDMENT			LAP SPLICE		
	COMPR	TENSION (LTE)		COMPR	TENSION (LTS)		HOOK	COMPR	TENSION (LTE)		COMPR	TENSION (LTS)	HOOK
BAR	(LCE)	TOP	OTHER	(LCS)	TOP	OTHER	(LDH)	(LCE)	TOP	OTHER	(LCS)	TOP	OTHER	(LDH)
#3	8	20	15	12	26	20	6	8	18	14	12	24	18	6
#4	10	26	20	15	34	26	8	9	25	19	15	32	25	7
#5	13	33	25	19	43	33	10	12	31	24	19	40	31	8
#6	15	40	30	23	51	40	12	14	37	28	23	48	37	10
#7	18	58	35	26	75	46	13	17	54	42	26	70	54	12
#8	20	66	41	30	86	53	15	19	62	47	30	80	62	13
#9	23	74	46	34	97	59	17	21	70	54	34	91	70	15
#10	26	84	52	38	109	67	19	24	78	60	38	102	78	17
#11	29	93	57	42	121	74	22	27	87	67	42	113	87	19

NOTES (PERTAINING TO TABLE):
1. TOP BARS ARE HORIZONTAL BARS THAT HAVE MORE THAN 12" OF FRESH CONCRETE CAST BELOW THEM.
2. ALL BARS THAT ARE NOT "TOP BARS" ARE "OTHER" BARS
3. ABBREVIATIONS:
   - LCE - COMPRESSION EMBEDMENT LENGTH
   - LTE - TENSION EMBEDMENT LENGTH
   - LCS - COMPRESSION LAP SPLICE LENGTH
   - LTS - TENSION LAP SPLICE LENGTH
   - LDH - HOOKED BAR TENSION EMBEDMENT LENGTH

NOTES (GENERAL):
1. STAGGER ALL SPLICES 12 db MIN, BUT NOT LESS THAN 12"
2. ALL DIMENSIONS INDICATED IN TABLE ARE IN INCHES
3. BARS GREATER THAN #11 SHALL BE MECHANICALLY SPLICED
4. ALL SPLICES SHALL BE WIRED IN CONTACT STACKED VERTICAL.

MULTIPLIERS:
ALL EMBEDMENT AND LAP SPLICE LENGTHS SHALL BE INCREASED AS REQ'D BY THE MULTIPLIERS BELOW. APPLY MULTIPLE MULTIPLIERS IF APPLICABLE.
1.3 – IF CONC CONTAINS LIGHT WEIGHT AGGREGATES
1.3 – IF EPOXY COATED REBAR USED

2 SPLICE & DEVELOPMENT SCHEDULE
NTS



STIRRUP AND TIE HOOK			
BAR SIZE	D (INCHES)	90° HOOK A OR G (INCHES)	135° HOOK A OR G (INCHES)
#3	1 ½	4	4
#4	2	4 ½	4 ½
#5	2 ½	6	5 ½

D= INSIDE DIAMETER OF BEND

1 TYPICAL STIRRUP AND TIE HOOK TYPES
NTS

FULLY SPRINKLERED

100% BID SET

No.	REVISION DESCRIPTION	DATE

12/19/2022 3:42:06 PM

CONSULTANTS:

STRUCTURAL / CIVIL ENGINEER
H2B, INC. (FIRM REG #: E-3405)
1225 N. LOOP WEST, SUITE 800
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MECH / ELEC / PLUMB / TECH ENGR
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FORCE PROTECT
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BELLEAIR, FL 33786
(502) 836-4232

ARCHITECT:

spur DESIGN

SPUR DESIGN, LLC
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Oklahoma City, OK 73109
(405) 842-6100

KS ARCH REG. NO. A-930, EXP. 12/31/2021
KS ENGR REG. NO. E-2586, EXP. 12/31/2021

STAMP:

SARAH J. BRISCOE LICENSED 28649 KANSAS PROFESSIONAL ENGINEER 12/21/22

Drawing Title
TYPICAL DETAILS - CONCRETE

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date **12/21/2022** | Checked SJB | Drawn ZAF

Project Number **589-704**

Building Number **26**

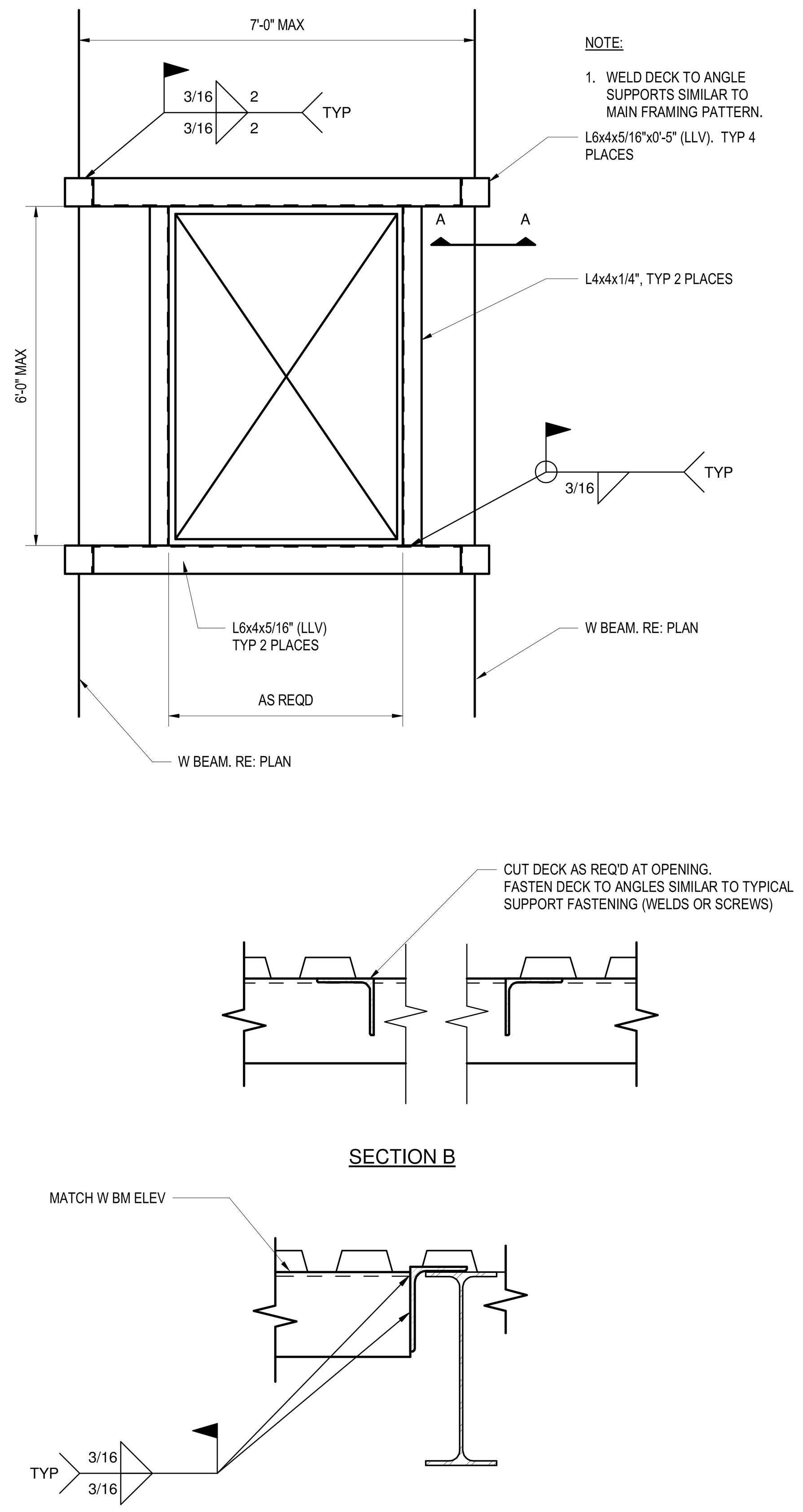
Drawing Number **SA030**

Drawing # 18 OF 190

Veterans Health Administration

VA U.S. Department of Veterans Affairs

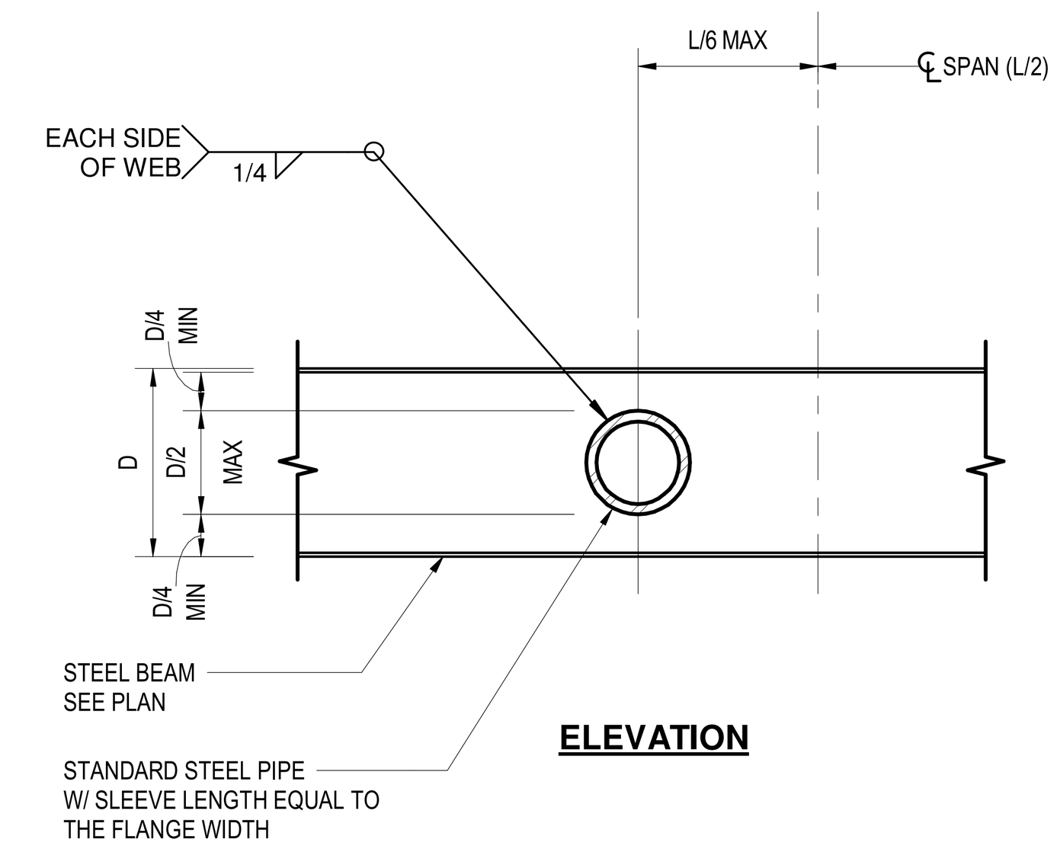
VA FORM 08-6231



NOTE:

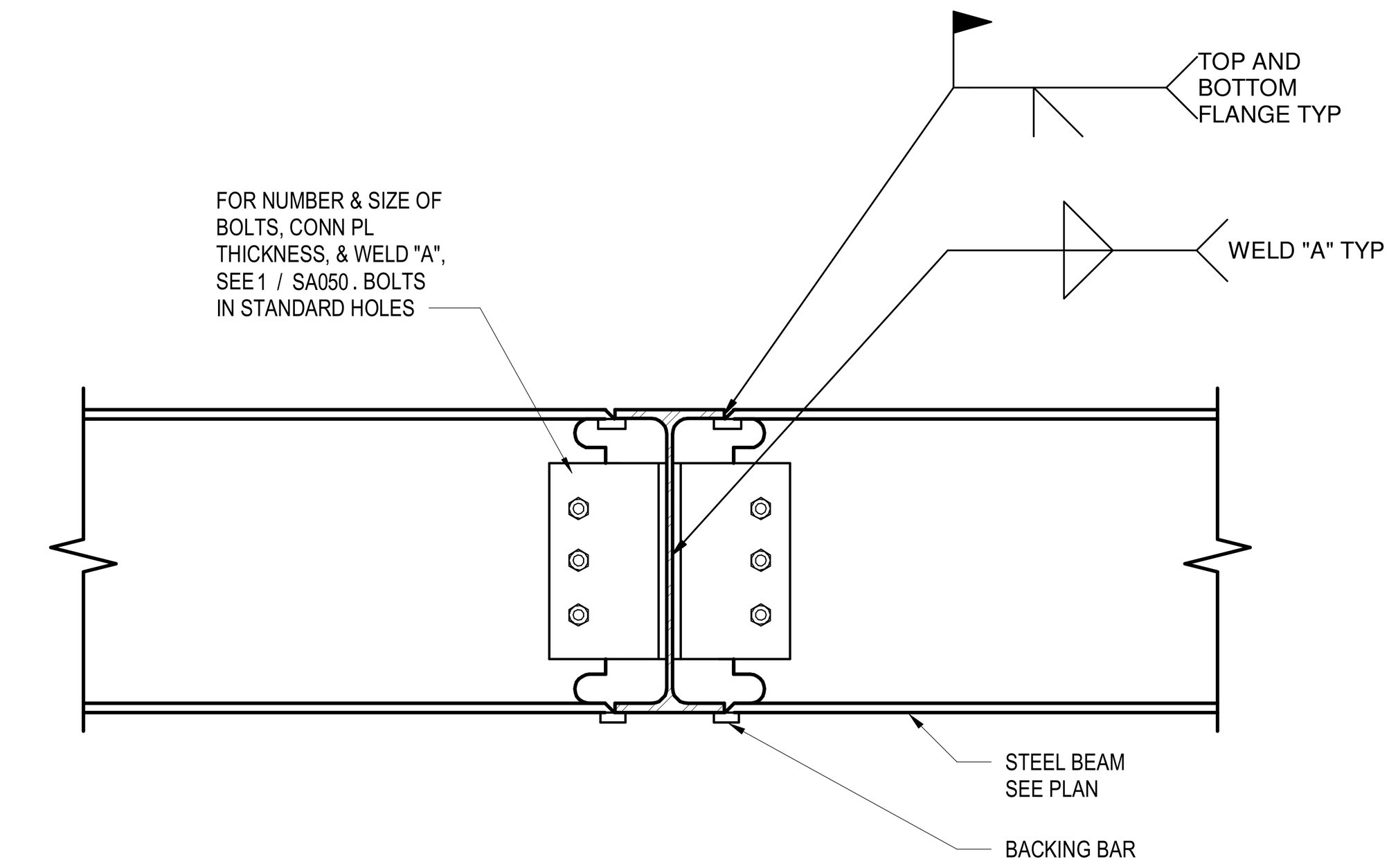
1. WELD DECK TO ANGLE SUPPORTS SIMILAR TO MAIN FRAMING PATTERN.

8 TYPICAL ROOF OPENING DETAIL
NTS



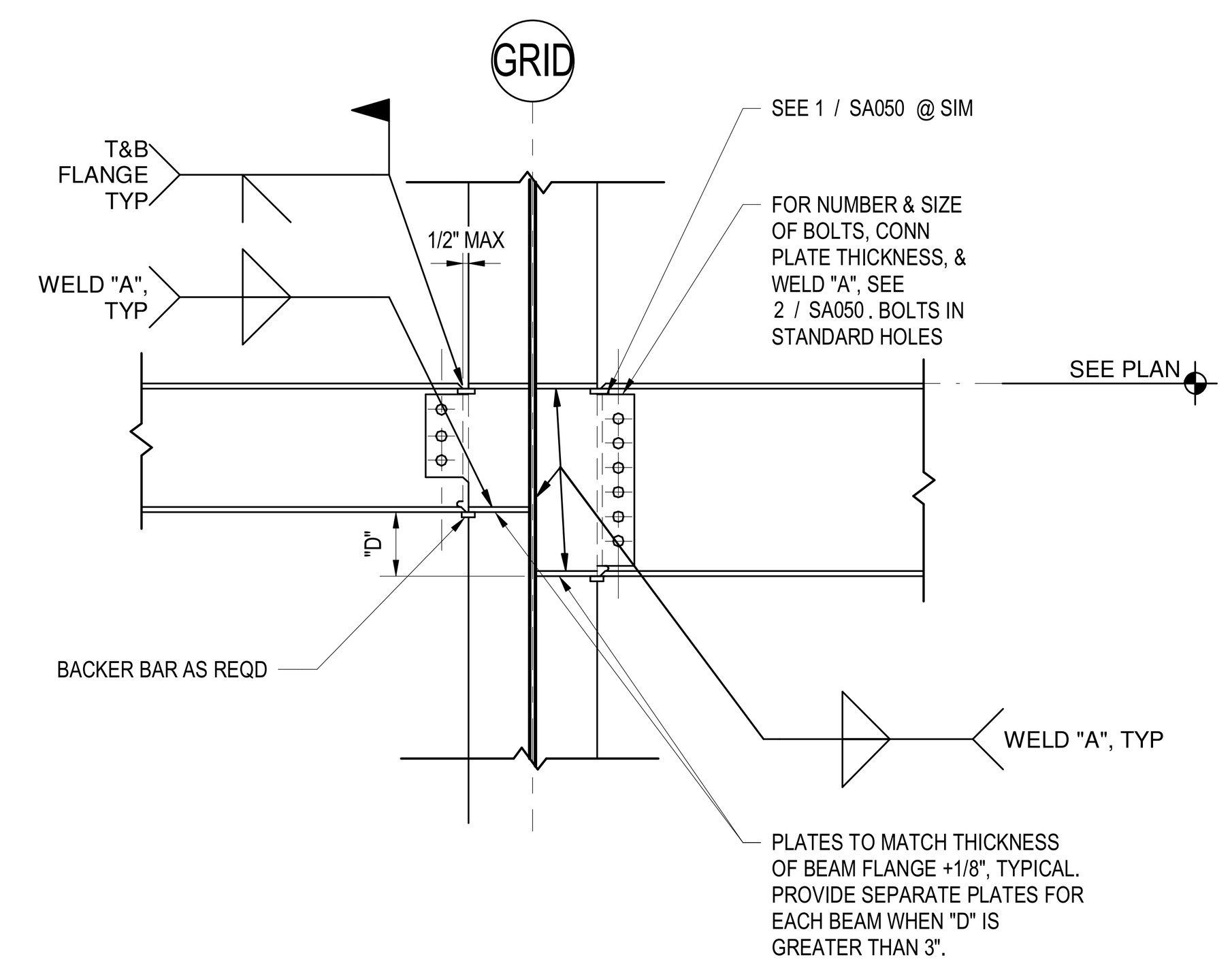
NOTES:
1. WHERE ROUND OPENINGS ARE 4" OR LESS REINFORCING SHOWN IS NOT REQUIRED
2. CONTRACTOR SHALL SUBMIT SIZE AND LOCATIONS OF ALL OPENINGS NOT SHOWN ON PLAN TO ENGINEER FOR APPROVAL PRIOR TO SUBMITTING OF STEEL SHOP DRAWINGS.

6 ROUND OPENING IN BEAM
NTS

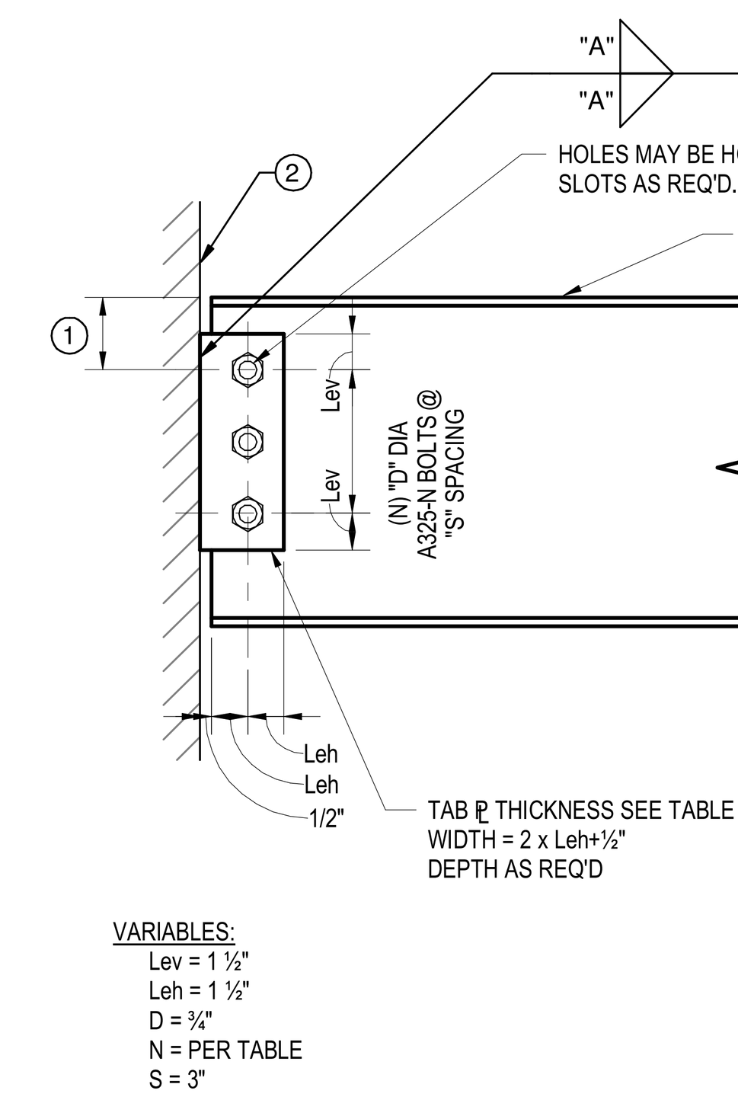


NOTE: BOLTS IN WEB CONNECTION SHALL BE TIGHTENED PRIOR TO WELDING OF FLANGES.

3 TYPICAL BM TO BM MOMENT CONNECTION
NTS



5 W BEAM TO W COL WEB MOMENT CONNECTION
NTS



VARIABLES:
Lev = 1 ½"
Leh = 1 ½"
D = ¾"
N = PER TABLE
S = 3"

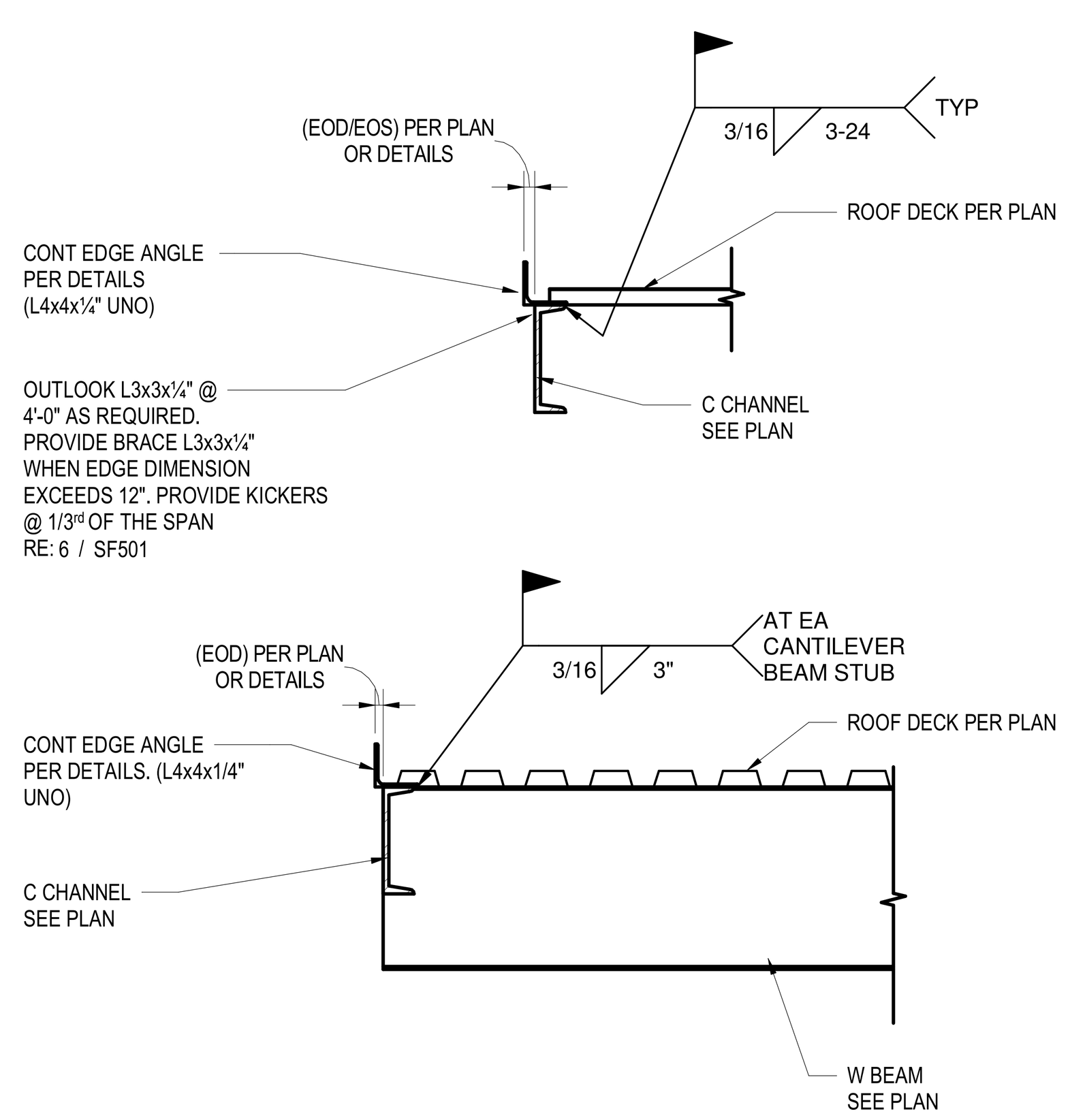
NOTES:

1. 3" DIM. TYP. EXCEPT AS RECOMMENDED BY AISC FOR W8 & SMALLER BEAMS OR FOR LARGER BEAMS.
2. FACE OF: BEAM WEB, COL. FLG., COL. WEB, OR EMBED PLATE.

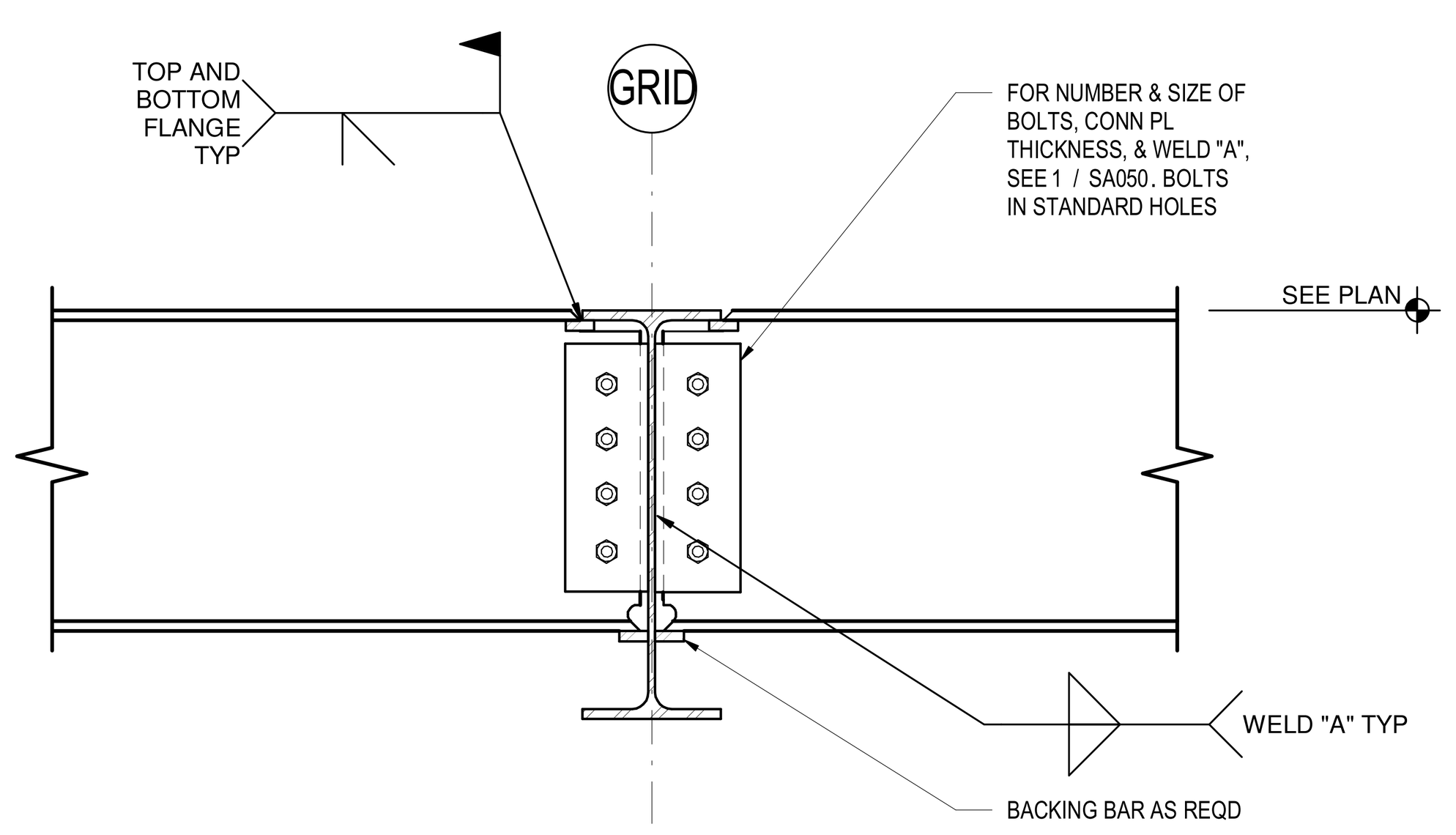
NOTE:
INFORMATION PROVIDED IN THIS DETAIL IS TO COMMUNICATE THE GENERAL INTENT OF THE CONNECTIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DESIGNING THE STEEL CONNECTIONS PER THE GENERAL NOTES.

BEAM	ℙ THICKNESS	N	WELD "A"
W8, W10, MC/C10	1/4"	2	3/16"
W12, W14	1/4"	3	3/16"
W16	1/4"	4	3/16"
W18, MC18	5/16"	5	1/4"
W24	5/16"	7	1/4"

2 TYPICAL SHEAR TAB CONNECTION
NTS

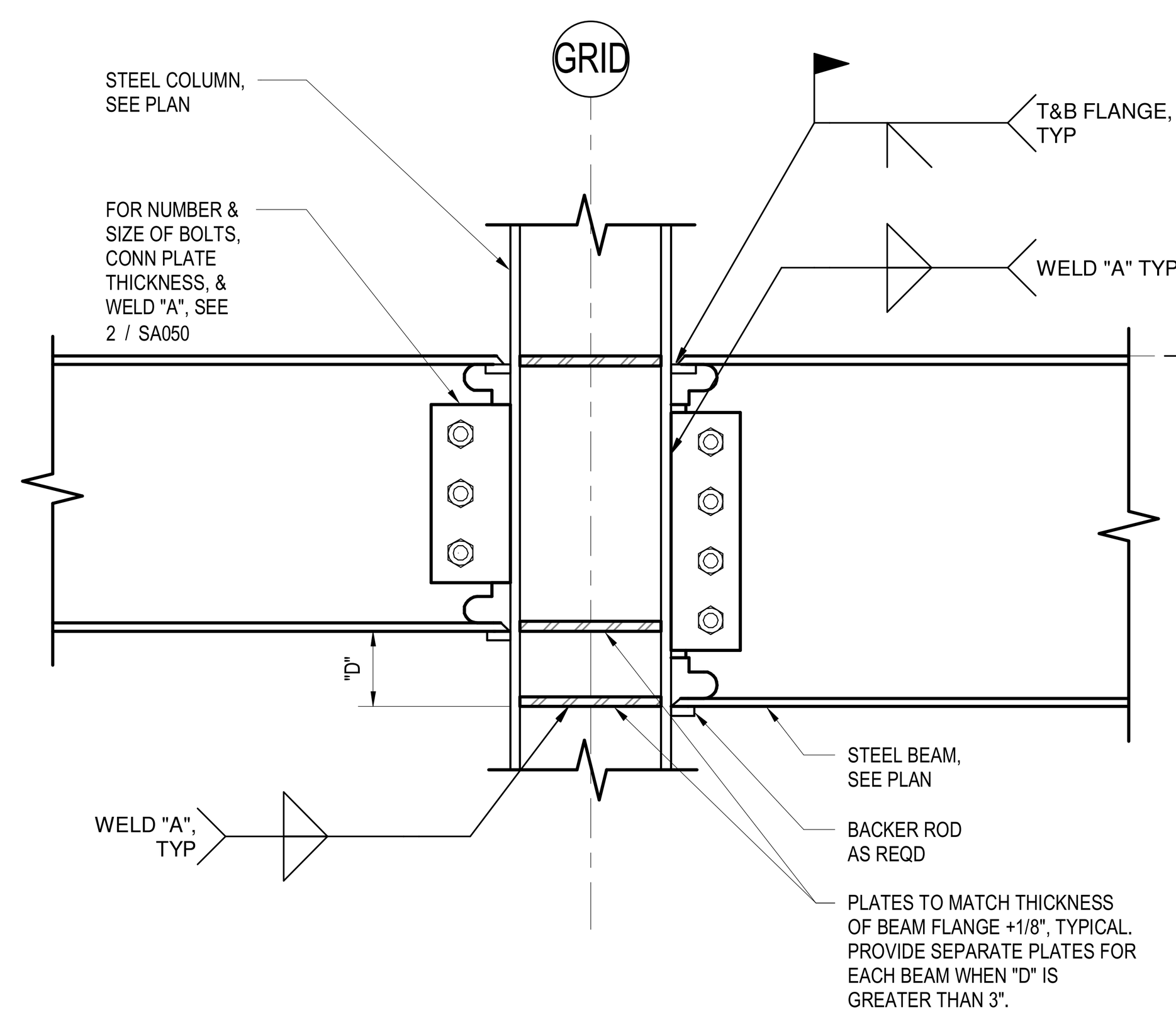


9 TYPICAL EDGE ANGLE @ ROOF
NTS

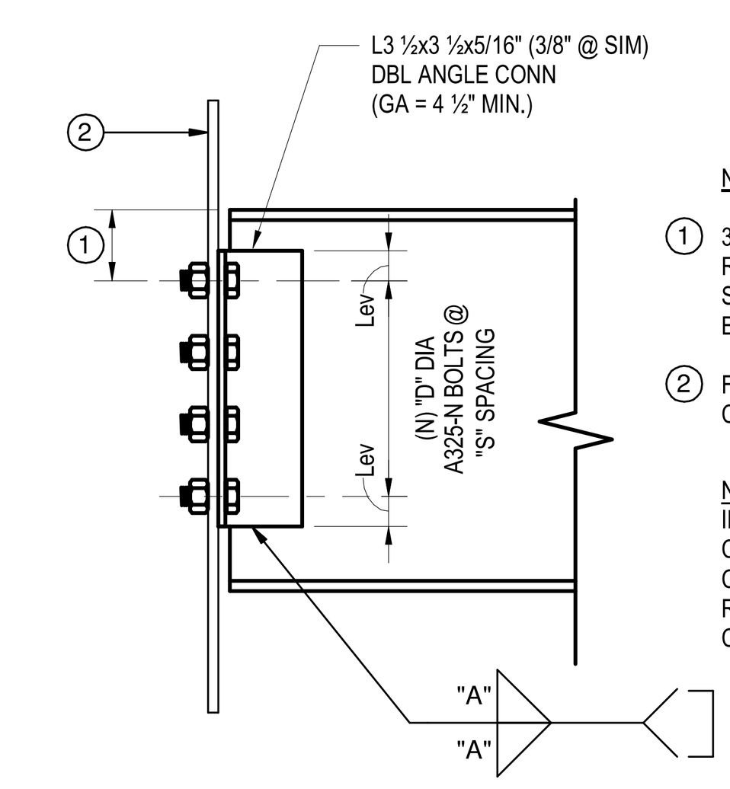


NOTE: BOLTS IN WEB CONNECTION SHALL BE TIGHTENED PRIOR TO WELDING OF FLANGES.

7 BEAM TO GIRDER MOMENT CONNECTION
NTS



4 TYPICAL FLANGE MOMENT CONNECTION
NTS



NOTES:

1. 3" DIM. TYP. EXCEPT AS RECOMMENDED BY AISC FOR W8 & SMALLER BEAMS OR FOR LARGER BEAMS.
2. FACE OF: BEAM WEB, COL. FLG., OR COL. WEB

NOTE:
INFORMATION PROVIDED IN THIS DETAIL IS TO COMMUNICATE THE GENERAL INTENT OF THE CONNECTIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DESIGNING THE STEEL CONNECTIONS PER THE GENERAL NOTES.

VARIABLES:
Lev = 1 1/2"
D = 3/4"
N = PER TABLE
S = 3"

BEAM	N	WELD "A"
W8, W10, C10	2	3/16"
W12, W14	3	3/16"
W16	4	1/4"
W18	5	1/4"
W24	7	1/4"

1 DOUBLE ANGLE CONNECTION
NTS

FULLY SPRINKLERED

100% BID SET

No.	REVISION DESCRIPTION	DATE

CONSULTANTS:

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H2B, INC. (FIRM REG #: E-3405)
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STAMP:
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LICENSED
28649
KANSAS
PROFESSIONAL ENGINEER
12/21/22
[illegible]

Drawing Title
TYPICAL DETAILS - STEEL

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

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WICHITA, KANSAS 67218

Date: **12/21/2022**
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589-704

Building Number
26

Drawing Number
SA050
Drawing # 19 OF 190

Veterans Health Administration

VA U.S. Department of Veterans Affairs

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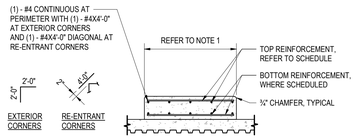
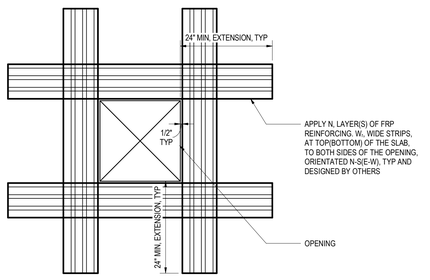
VA FORM 08-6231



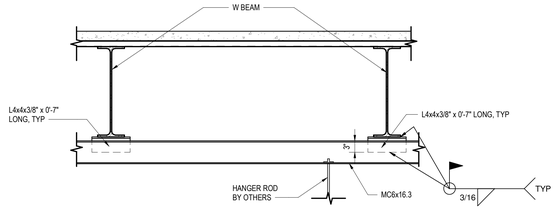
HOUSEKEEPING PAD REINFORCEMENT SCHEDULE		
PAD THICKNESS	TOP REINFORCEMENT	BOTTOM REINFORCEMENT
1'-4"	#3@12" EACH WAY	NONE

NOTE:
1. REFER TO ARCHITECTURAL, STRUCTURAL, OR MEP DRAWING FOR HOUSEKEEPING PAD PLAN DIMENSIONS AND THICKNESS (4" MINIMUM THICKNESS).
2. CONTRACTOR SHALL COORDINATE DIMENSIONS AND OTHER SPECIAL REQUIREMENTS WITH EQUIPMENT MANUFACTURERS AND PROVIDE WHERE REQUIRED WHETHER SHOWN ON STRUCTURAL DRAWING OR NOT.
3. PROVIDE ADDITIONAL LAYER OF REINFORCING FOR PAD THICKNESS GREATER THAN 18".

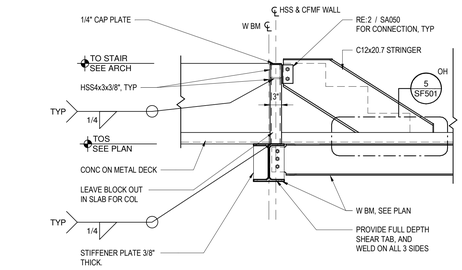
9 TYPICAL HOUSEKEEPING PAD OVER SLAB ON DECK
NTS



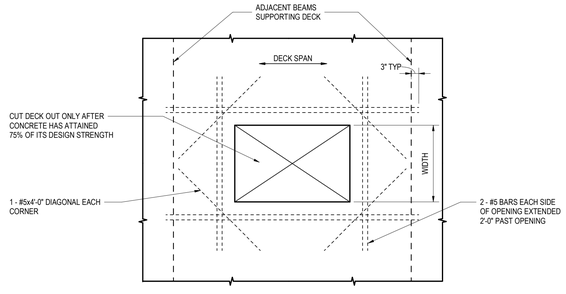
8 TYPICAL FRP STRENGTHENING @ OPENING IN EXISTING SLAB
NTS



7 TYPICAL KITCHEN HOOD SUPPORT
NTS

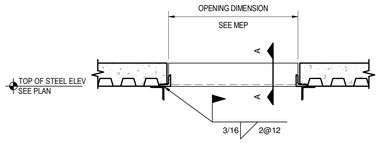


6 STAIR STRINGER CONNECTION DETAIL
NTS

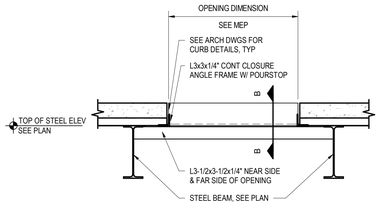


THIS DETAIL APPLIES TO OPENINGS WHERE THE "WIDTH" IS 10" TO 2'-0" OMIT #5 BARS AT OPENINGS LESS THAN 10" IN WIDTH. OPENINGS GREATER THAN 2'-0" MUST BE FRAMED WITH STEEL BEAMS.

5 TYPICAL REINFORCEMENT DETAIL OF OPENINGS IN FLOOR SLABS
NTS

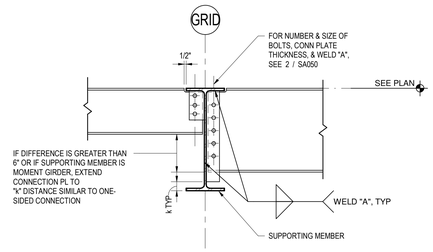


SECTION B-B

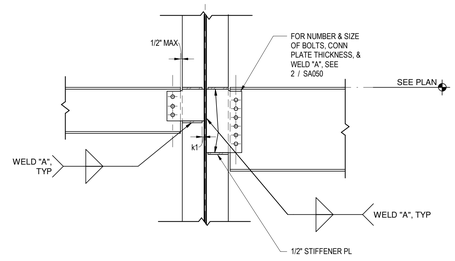


SECTION A-A

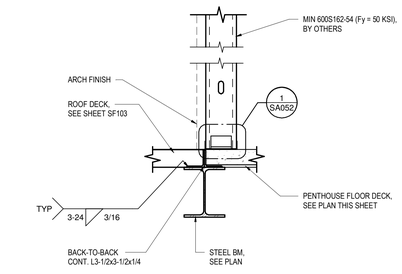
4 TYPICAL FRAMING AT FLOOR OPENING
NTS



3 BEAM TO GIRDER - TWO SIDED DETAIL
NTS



2 BEAM TO COLUMN WEB DETAIL
NTS



1 ROOF DECK TRANSITION DETAIL
NTS

FULLY SPRINKLERED

100% BID SET

No.	REVISION DESCRIPTION	DATE

12/19/2022 3:42:10 PM

VA FORM 08-6231

CONSULTANTS:

STRUCTURAL / CIVIL ENGINEER
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KS ENGR REG. NO. E-2586, EXP. 12/31/2021

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SARAH J. BRISCOE LICENSED 28649 KANSAS PROFESSIONAL ENGINEER 12/21/22

Drawing Title
TYPICAL DETAILS - STEEL

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date **12/21/2022** | Checked Checker | Drawn Author

Project Number **589-704**

Building Number **26**

Drawing Number **SA051**

Drawing # 20 OF 190

Veterans Health Administration

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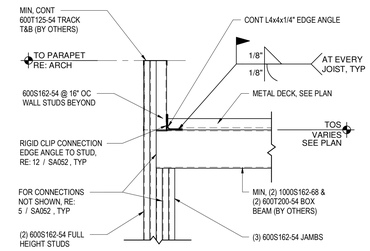
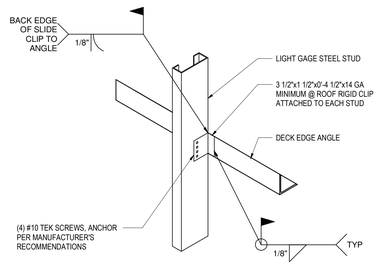


11 CFMF CONNECTION DETAIL @ WALL
NTS



12 RIGID CLIP DETAIL
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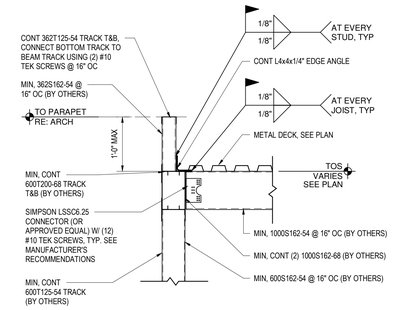
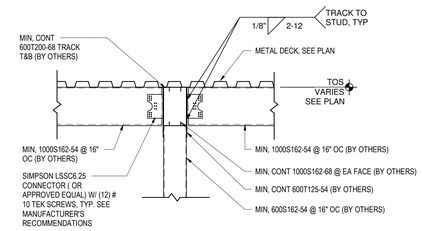


10 CFMF CONNECTION DETAIL @ WALL
NTS

TOP TRACK SPLICE

9 TYPICAL STEEL STUD WALL FRAMING
NTS



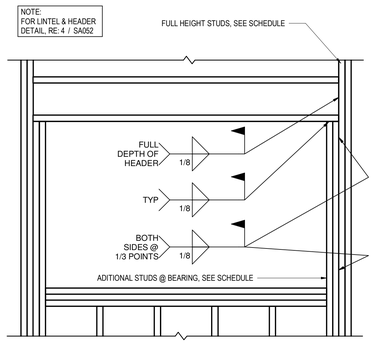
8 CFMF CONNECTION DETAIL @ WALL
NTS

LIGHT-GAGE LINTEL SCHEDULE					
MARK	OPENING WIDTH	SIZE	STUDS AT BEARING	FULL HEIGHT STUDS	SILL PLATE STUDS
SL-1	<=11'-0"	(2) 600S162-54 (2) 600T125-54	(3) 600S162-54	(2) 600S162-54	-

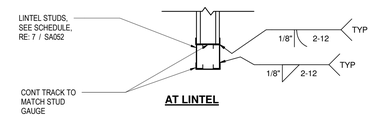
7 LIGHTGAGE LINTEL SCHEDULE
NTS

WELD SCHEDULE - COLD-FORMED STEEL FRAMING	
LOCATION	FILLET WELD SIZE & SPACING
STUD TO TRACK	1/8" AT EACH FLANGE
STUD TO STUD	1/8"X1" @ 12" OC AT EACH FLANGE
BUILT-UP LINTEL	1/8"X1" @ 12" OC AT EACH FLANGE OF TRACK
LINTEL TO SUPPORT STUDS	1/8" FOR FULL LENGTH OF BEARING, EACH SIDE OF BEAM
TRACK TO TRACK	1/8"X1" @ 12" OC EACH SIDE
TRACK TO STUD AT JAMB	1/8"X3" MINIMUM
LEDGER TO STUD	1/8" TOP & BOTTOM EACH STUD
DIAGANOL KICKER	1/8"X3" TOP & BOTTOM EACH STUD EA SIDE

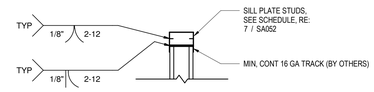
6 WELD SCHEDULE
NTS



5 TYPICAL OPENING FRAMING
NTS

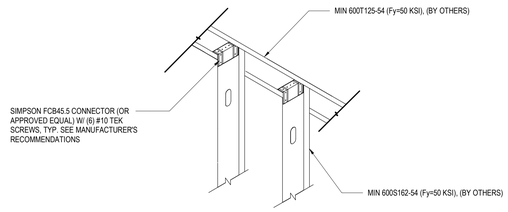


AT LINTEL

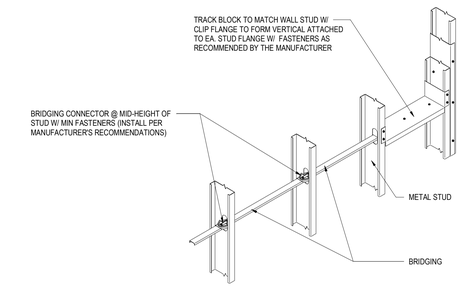


AT SILL

4 LINTEL & SILL CONN
NTS



3 TOP OF WALL FRAMING DETAIL
NTS



2 WALL BRIDGING DETAIL
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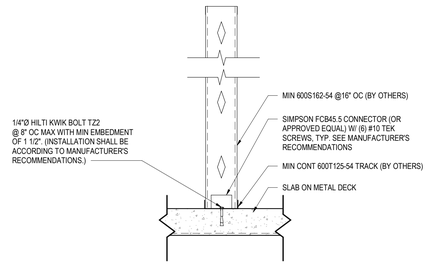

1 TRACK TO SLAB ON METAL DECK DETAIL
NTS

FULLY SPRINKLERED

100% BID SET

No.	REVISION DESCRIPTION	DATE

12/19/2022 3:42:12 PM

VA FORM 08-6231

CONSULTANTS:

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KS ENGR REG. NO. E-2586, EXP. 12/31/2021

STAMP:

SHAWN J. BRISCHE LICENSED 28649 KANSAS PROFESSIONAL ENGINEER 12/21/22

Drawing Title
TYPICAL DETAILS - CFMF

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date **12/21/2022** | Checked SJB | Drawn ZAF

Project Number **589-704**

Building Number **26**

Drawing Number **SA052**

Drawing # 21 OF 190

Veterans Health Administration

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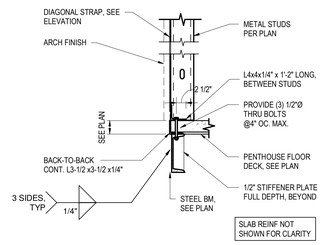
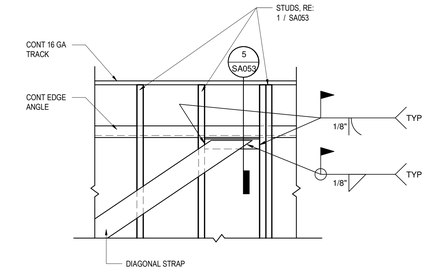
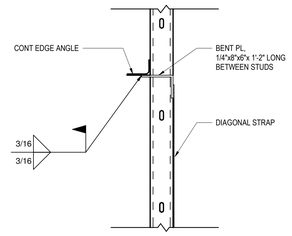


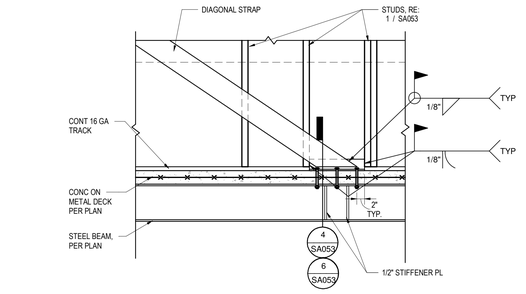
6 HOLD-DOWN AT CHANNEL DETAIL
NTS



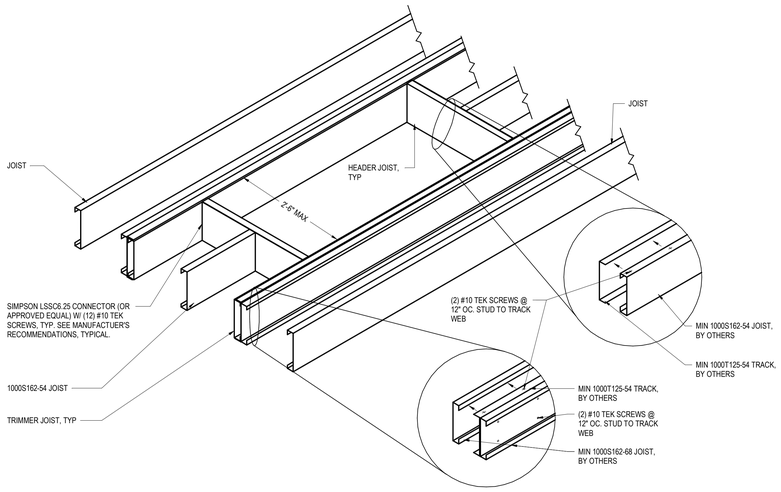
3 STRAP TO EDGE ANGLE DETAIL
NTS



5 STRAP CONNECTION DETAIL
NTS



2 STRAP TO STEM WALL DETAIL
NTS



7 CFMF ROOF OPENING DETAIL
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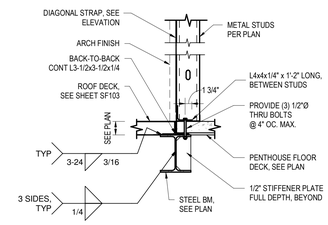
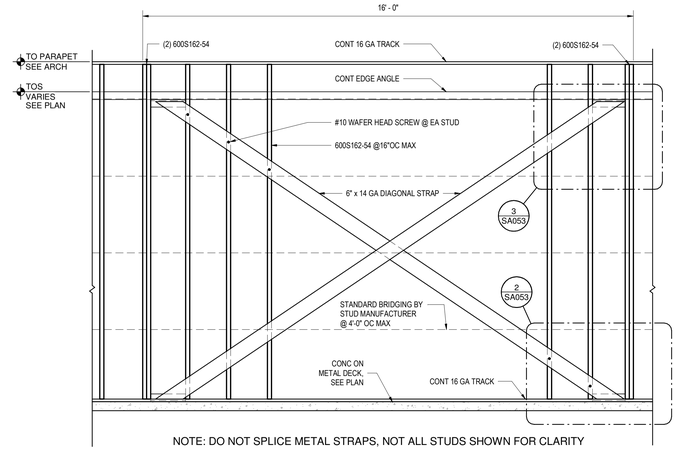


4 HOLD-DOWN AT W BEAM DETAIL
NTS



NOTE: DO NOT SPLICE METAL STRAPS, NOT ALL STUDS SHOWN FOR CLARITY

1 STRAP BRACE ELEVATION
NTS

FULLY SPRINKLERED

100% BID SET

No.	REVISION DESCRIPTION	DATE

CONSULTANTS:

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KS ENGR REG. NO. E-2586, EXP. 12/31/2021

STAMP:

SARAH J. BRISCOE LICENSED 28649 KANSAS PROFESSIONAL ENGINEER 12/21/22

Drawing Title
TYPICAL DETAILS - CFMF

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date **12/21/2022** | Checked Checker | Drawn Author

Project Number **589-704**

Building Number **26**

Drawing Number **SA053**

Drawing # 22 OF 190

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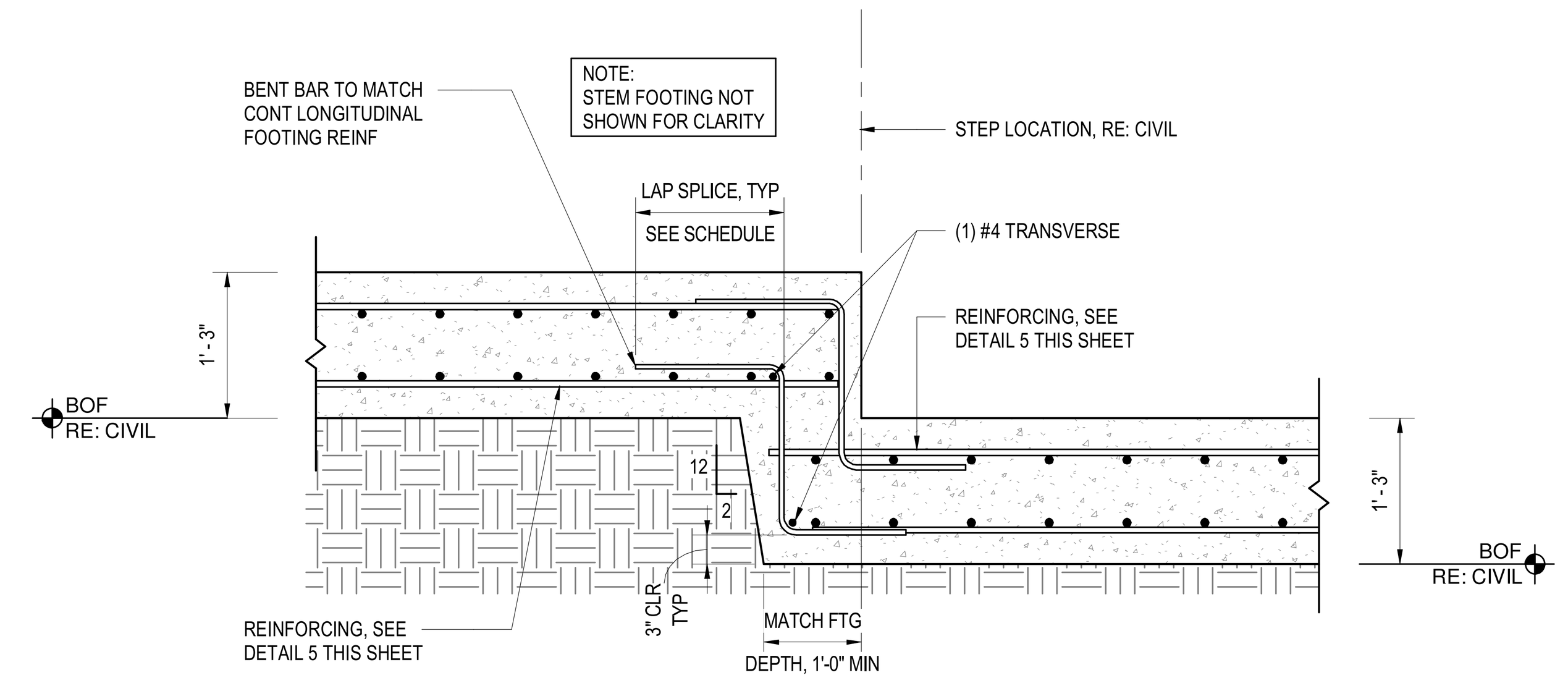
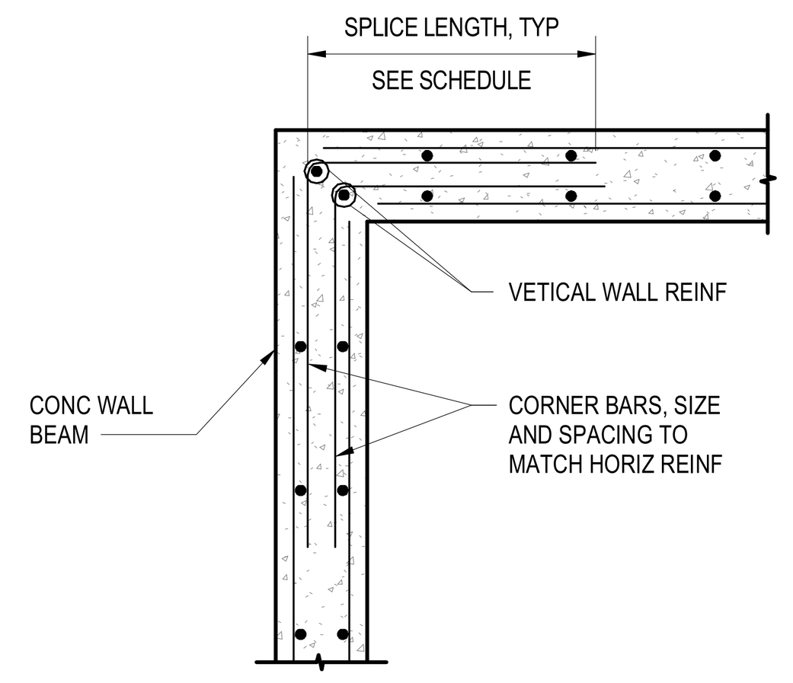
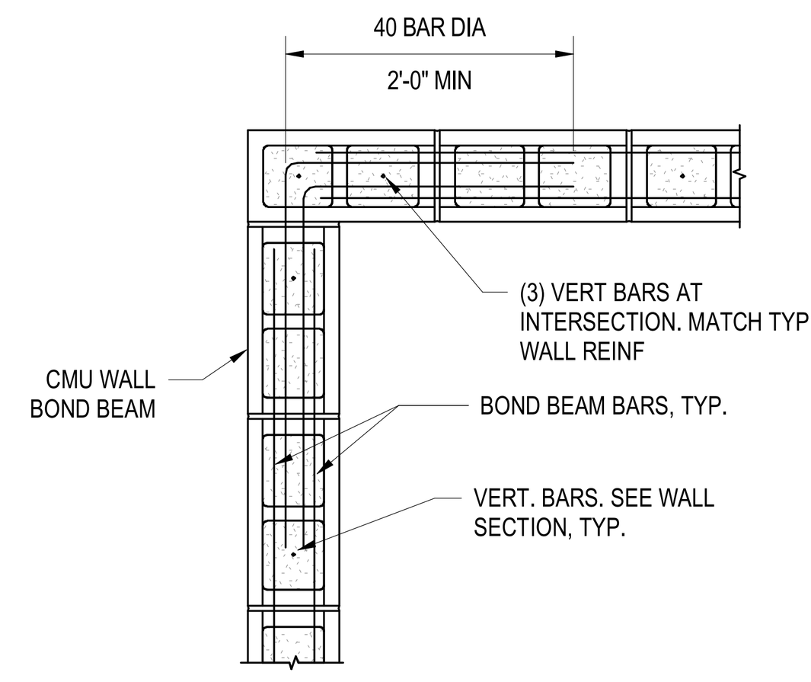


6 TYPICAL STEPPED FOOTING DETAIL
NTS



4 TYPICAL WALL FOOTING CORNER
NTS

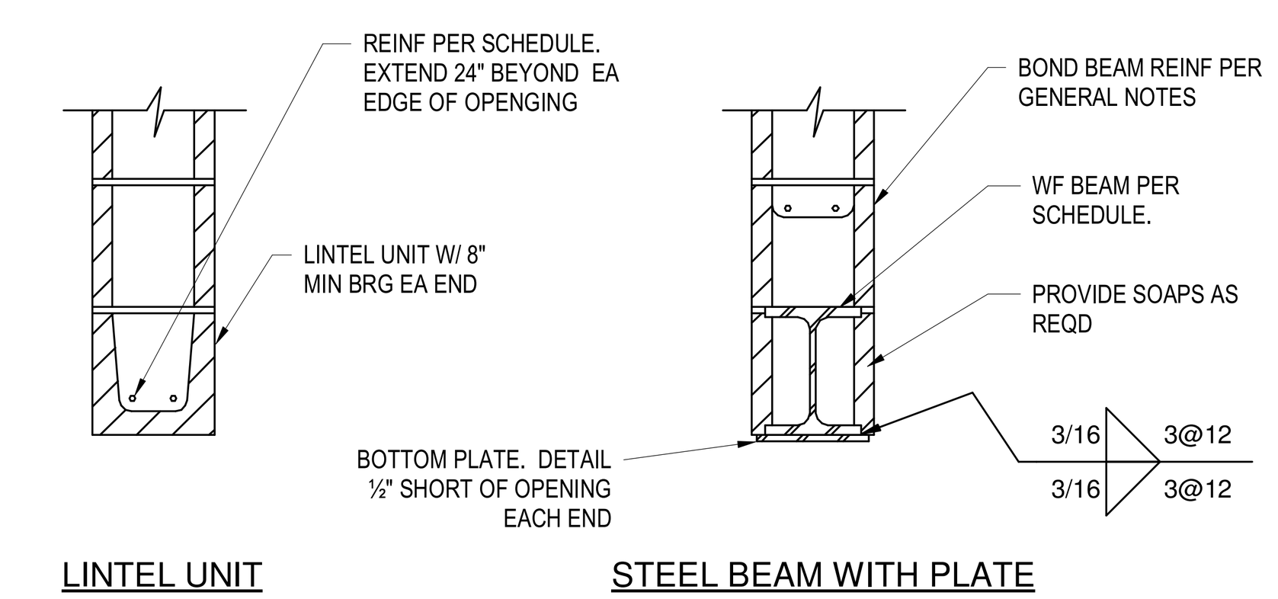


3 TYPICAL CMU BOND BEAM CORNERS
NTS

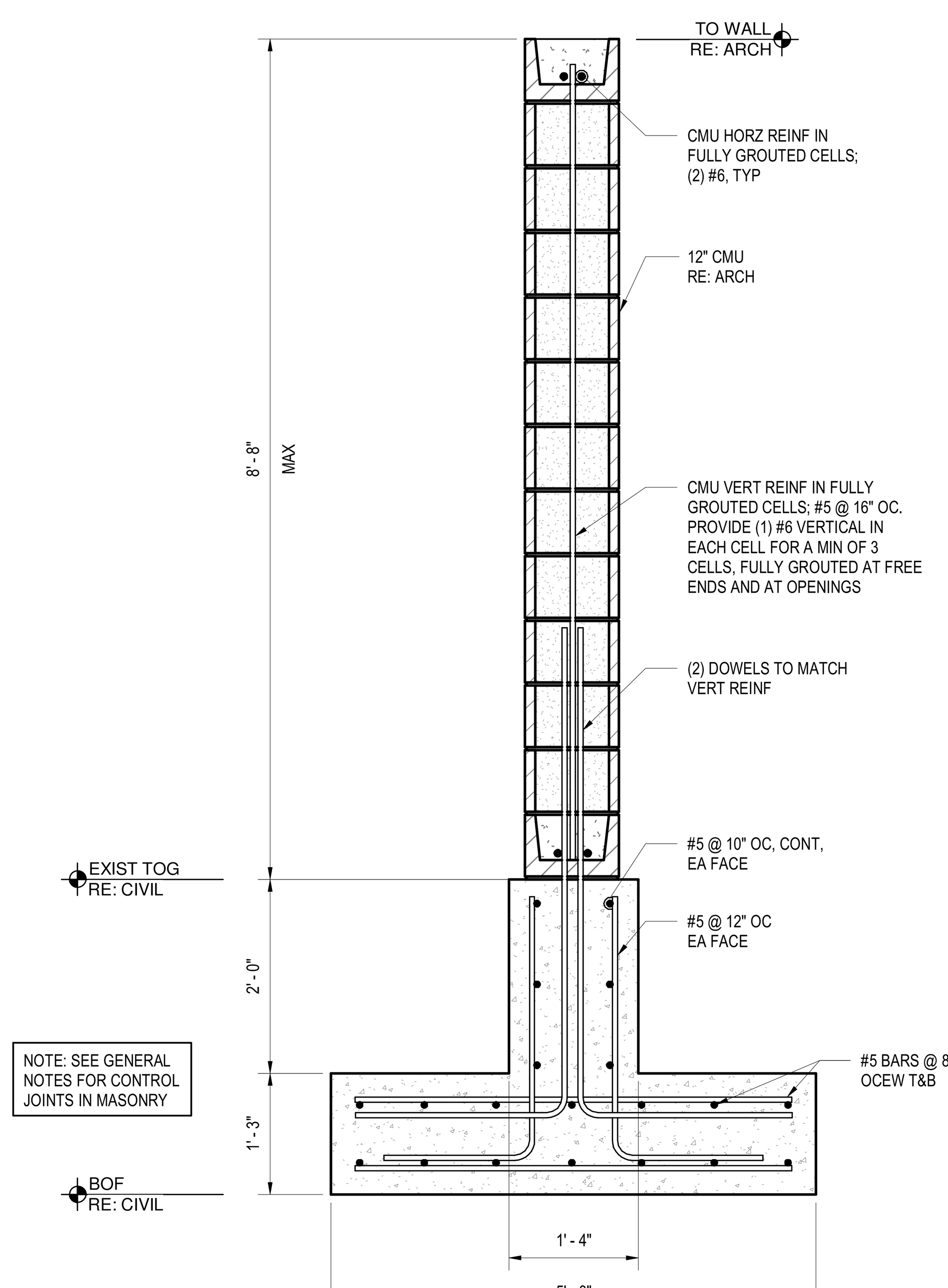
8" CMU LINTEL SCHEDULE	
MAX OPNG "W"	CMU LINTEL
6'-0"	8" HIGH LINTEL UNIT W/ 2-#5 HORZ BOTTOM
8'-0"	16" HIGH LINTEL UNIT W/ 2-#5 HORZ BOTTOM
10'-0"	W8X15 BEAM W/ 1/4" X 7" BOTTOM PLATE
12'-0"	W8X21 BEAM W/ 1/4" X 7" BOTTOM PLATE
16'-0"	W16X31 BEAM W/ 1/4" X 7" BOTTOM PLATE

NOTE:

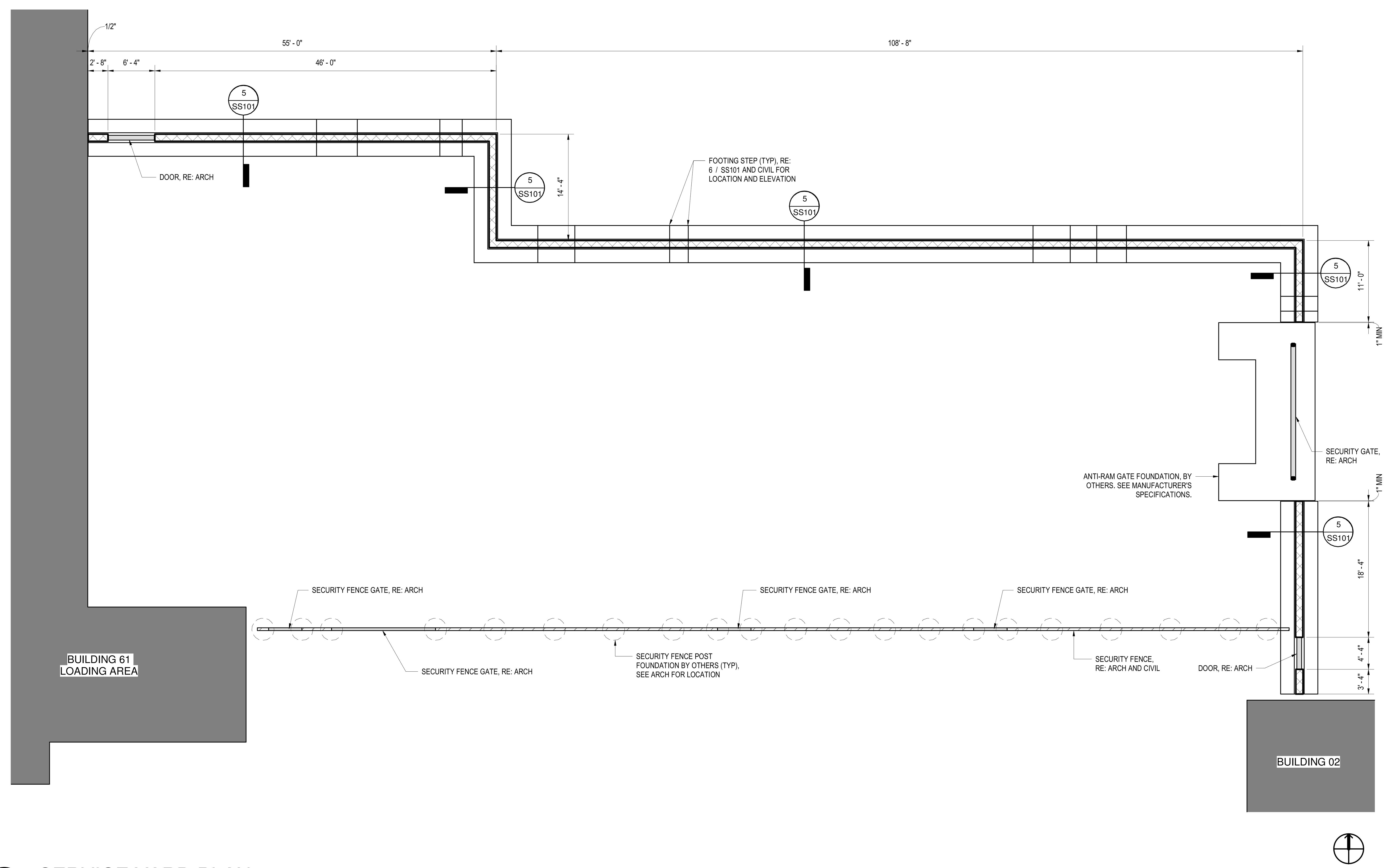
AT EACH END OF A STEEL LINTEL, PROVIDE A PLATE 3/8" x 7" x 0'-8" W/ (2) 1/2" DIAMETER x" 4" HEADED STUDS. FIELD WELD BEAM BOTTOM FLANGE TO PLATE 3/16" x 3" LONG EACH SIDE AND EACH END.



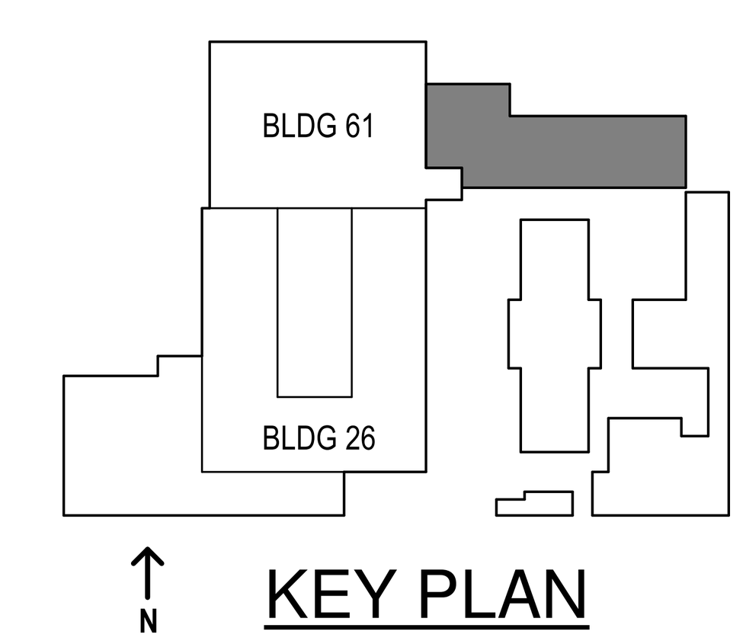
2 TYPICAL LINTEL SCHEDULE
NTS



5 SERVICE YARD WALL FOUNDATION
NTS



1 SERVICE YARD PLAN
1/8" = 1'-0"



KEY PLAN

FULLY SPRINKLERED

100% BID SET

No.	REVISION DESCRIPTION	DATE

CONSULTANTS:

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KS ENGR REG. NO. E-2586, EXP. 12/31/2021

STAMP:
SARAH J. BRISCOE
LICENSED
28649
KANSAS
PROFESSIONAL ENGINEER
12/21/22
[illegible]

Drawing Title
SERVICE YARD PLAN/DETAILS

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date: **12/21/2022** | Checked: SJB | Drawn: ZAF

Project Number: **589-704**

Building Number: **26**

Drawing Number
SS101
Drawing # 23 OF 190

Veterans Health Administration

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VA FORM 08-6231

three inches = one foot
one and one half inches = one foot
one inch = one foot
three quarters inch = one foot
one half inch = one foot
three eighths inch = one foot
one quarter inch = one foot
one eighth inch = one foot

GENERAL DEMOLITION SHEET NOTES

A. THE CONTRACTOR IS FULLY RESPONSIBLE FOR THE MEANS AND METHODS OF DEMOLITION AND THE INTEGRITY AND STABILITY OF THE EXISTING STRUCTURE DURING DEMOLITION UNTIL THE WORK IS COMPLETED. THE CONTRACTOR WILL PROVIDE SHORING IN REQUIRED LOCATIONS WHERE EXISTING CONSTRUCTION TO REMAIN WILL BE AFFECTED BY DEMOLITION AND RELATED ACTIVITIES. TEMPORARY AND PERMANENT SHORING AND UNDERPINNING WORK AND CALCULATIONS WILL BE DESIGNED , SIGNED AND SEALED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF KANSAS AND SUBMITTED TO THE DESIGN ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO START OF DEMOLITION WORK.

B. ALL DEMOLITION WORK WILL BE COORDINATED WITH EXISTING FIELD CONDITIONS, ACTIVITY DESCRIBED IN THE DEMOLITION AND CONSTRUCTION DRAWINGS AND OTHER ACTIVITIES ON-SITE. REFER TO THE ARCHITECTURAL, CIVIL, MECHANICAL AND ELECTRICAL DEMOLITION AND CONSTRUCTION DRAWINGS. IN THE EVENT OF CONFLICTS, THE CONTRACTOR WILL NOTIFY THE GOVERNMENT IMMEDIATELY.

C. ALL EXISTING FRAMING INDICATED ON THESE DRAWINGS IS FOR REFERENCE ONLY AND IS TO BE FIELD VERIFIED BY THE CONTRACTOR. VERIFY THE EXACT EXTENT OF DEMOLITION AT THE SITE. DETERMINE THE NATURE AND EXTENT OF DEMOLITION THAT WILL BE NECESSARY BY COMPARING THE CONTRACT DOCUMENTS WITH THE EXISTING CONSTRUCTION. COORDINATE ALL DEVIATIONS WITH THE GOVERNMENT.

D. THE CONTRACTOR IS RESPONSIBLE FOR REPAIRS TO ANY STRUCTURAL ELEMENTS WHICH ARE TO REMAIN AND THAT HAVE BEEN DAMAGED DURING DEMOLITION OPERATIONS TO THE COMPLETE SATISFACTION OF THE GOVERNMENT. THE REPAIRS WILL BE AT NO EXPENSE TO THE GOVERNMENT. ALL REPAIR WORK WILL BE DESIGNED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF MARYLAND AND SUBMITTED TO THE DESIGN ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO COMMENCING REPAIR WORK.

E. THE CONTRACTOR WILL USE QUALIFIED, EXPERIENCED PERSONNEL FOR DEMOLITION AND REMOVAL OPERATIONS. PERFORM DEMOLITION AND REMOVAL OPERATIONS IN A CAREFUL AND ORDERLY MANNER TO PREVENT HAZARDS TO PERSONS, DAMAGE TO PROPERTY, AND THE SPREADING OF DUST AND DEBRIS.

F. DO NOT PERMIT PORTIONS OF THE STRUCTURE TO FALL OR DEBRIS TO DROP EXCEPT BY METHODS WHICH WILL ENSURE THE INTEGRITY OF THE STRUCTURE.

G. PRIOR TO THE START OF WORK, VERIFY THAT THE SCOPE OF DEMOLITION INDICATION ON THE CONTRACT DOCUMENTS WILL NOT DAMAGE, CUT OR DISRUPT SERVICE OF ANY MECHANICAL SYSTEM, ELECTRICAL SYSTEM OR UTILITY TO THIS OR ANY OTHER STRUCTURE.

H. DO NOT REMOVE MORE OF THE EXISTING STRUCTURE THAN INDICATED ON THE CONTRACT DOCUMENTS. DO NOT DAMAGE, MAR, CUT OR DEFACE THE REMAINING STRUCTURE OR MATERIALS TO BE REUSED.

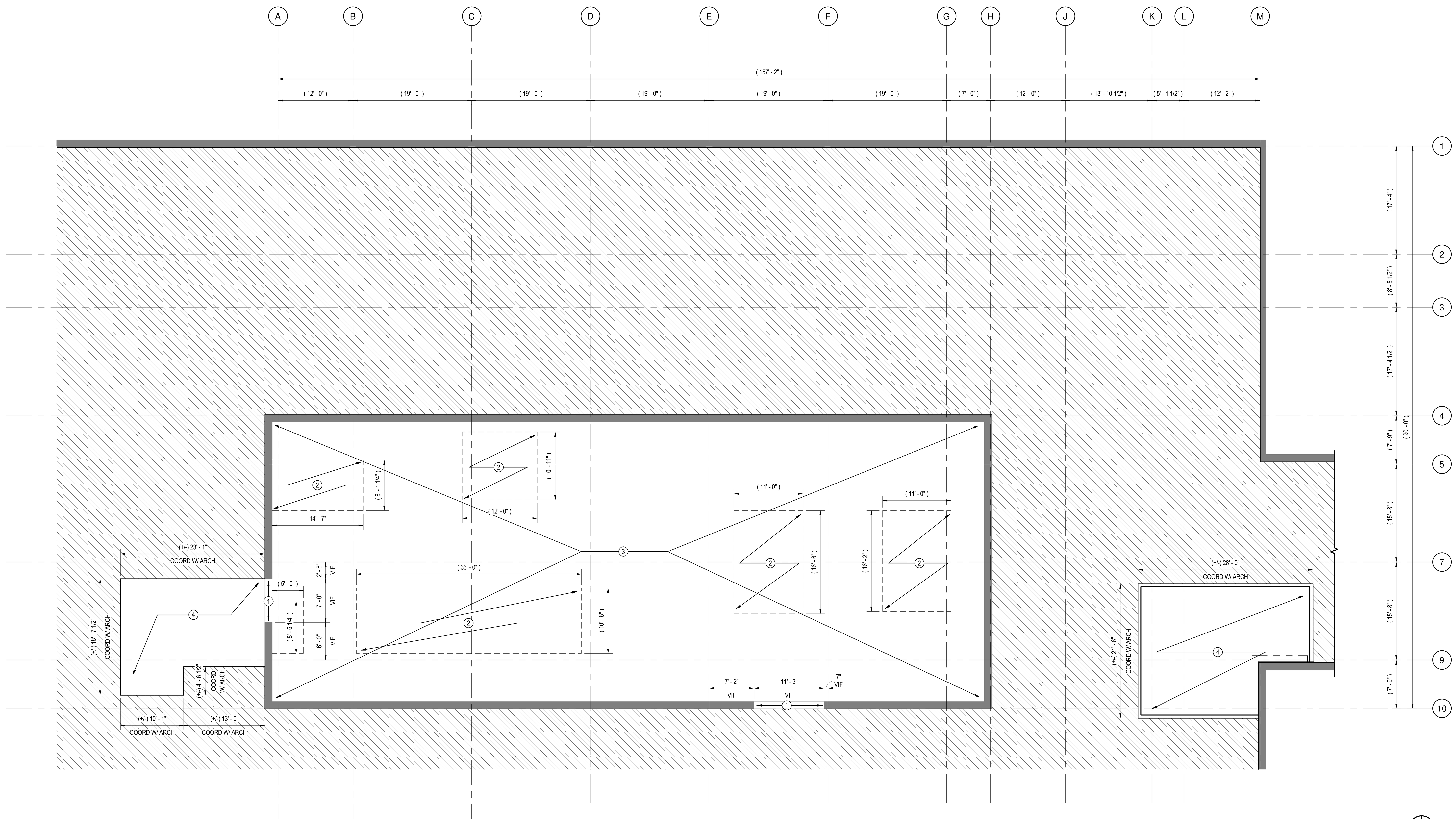
I. THE CONTRACTOR WILL INCLUDE IN THEIR BID THE COST OF REMOVING DEMOLISHED MATERIALS FROM THE SITE IN ACCORDANCE WITH ALL APPLICABLE LAWS, CODES AND REGULATIONS.

J. WHERE NEW OPENINGS IN EXISTING SLABS OR WALLS ARE TO BE CREATED, THE DEMOLITION CONTRACTOR WILL CORE HOLES AT THE OUTSIDE CORNERS OF THE NEW OPENING PRIOR TO DEMOLITION; THE DEMOLITION CONTRACTOR WILL CORE HOLES AT THE OUTSIDE CORNERS OF THE NEW OPENING PRIOR TO DEMOLITION. SAW CUT AND DEMOLISH SLAB OR WALL ONLY AFTER THE INSTALLATION OF ALL REQUIRED NEW STRUCTURAL FRAMING, SHORING, UNDERPINNING AND/OR REINFORCEMENT IS IN PLACE AND SECURED. SAW CUTTING WILL BE STRAIGHT AND WILL NOT EXTEND INTO THE EXISTING SLAB OR WALL TO REMAIN OR BEYOND THE CORED HOLES AT THE CORNERS OF A NEW OPENING.

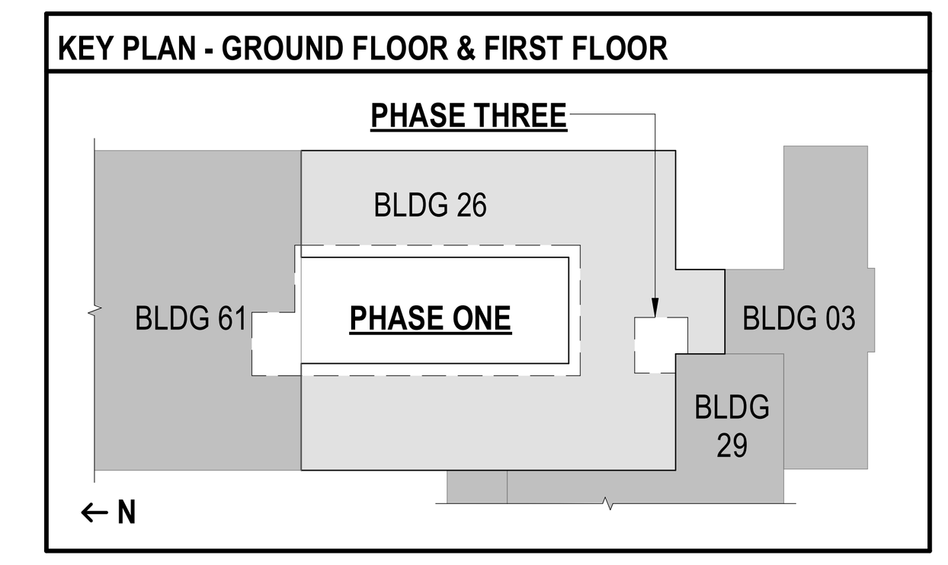
K. (X'-X") INDICATES EXIST DIMENSIONS OR ELEVATIONS. GENERAL CONTRACTOR TO FIELD VERIFY EXIST CONDITIONS, ELEVATIONS AND DIMENSIONS.

(#) GROUND FLOOR DEMO PLAN NOTES:

1. NEW/EXTENDED OPENING IN EXISTING WALL. COORD SIZE AND LOCATION WITH ARCH AND EXISTING CONDITIONS. RE: SF301 FOR ELEVATIONS.
2. EXIST CONCRETE MECHANICAL PAD TO BE REMOVED.
3. EXISTING SOIL TO BE REMOVED AND SUBGRADE COMPACTED AND TREATED PRIOR TO PLACEMENT OF STRUCTURAL FILL MATERIAL, PER THE GENERAL NOTES AND GEOTECHNICAL REPORT.
4. SAW CUT, DEMOLISH AND REMOVE EXIST CONCRETE SLAB. FOR DEMOLITION AT/ALONG EXTERIOR FOUNDATION WALL, CONTRACTOR IS TO ENSURE THE EXIST FOUNDATION BELOW NEW SLAB IS NOT DAMAGED DURING DEMOLITION. COORDINATE EXTENTS w/ ARCH AND MECHANICAL.



1 EXISTING 26 FOUNDATION DEMO PLAN
1/8" = 1'-0"



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100% BID SET

No.	REVISION DESCRIPTION	DATE

CONSULTANTS:

STRUCTURAL / CIVIL ENGINEER
H2B, INC. (FIRM REG #: E-3405)
1225 N. LOOP WEST, SUITE 800
HOUSTON, TX 77008
(713) 864-2900

MECH / ELEC / PLUMB / TECH ENGR
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KS ARCH REG. NO. A-930, EXP. 12/31/2021
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STAMP:
SARAH J. BRISCOE LICENSED 28649 KANSAS PROFESSIONAL ENGINEER
12/21/22
[illegible]

Drawing Title
GROUND FLOOR DEMO PLAN

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date	Checked	Drawn
12/21/2022	SJB	ZAF

Project Number
589-704

Building Number
26

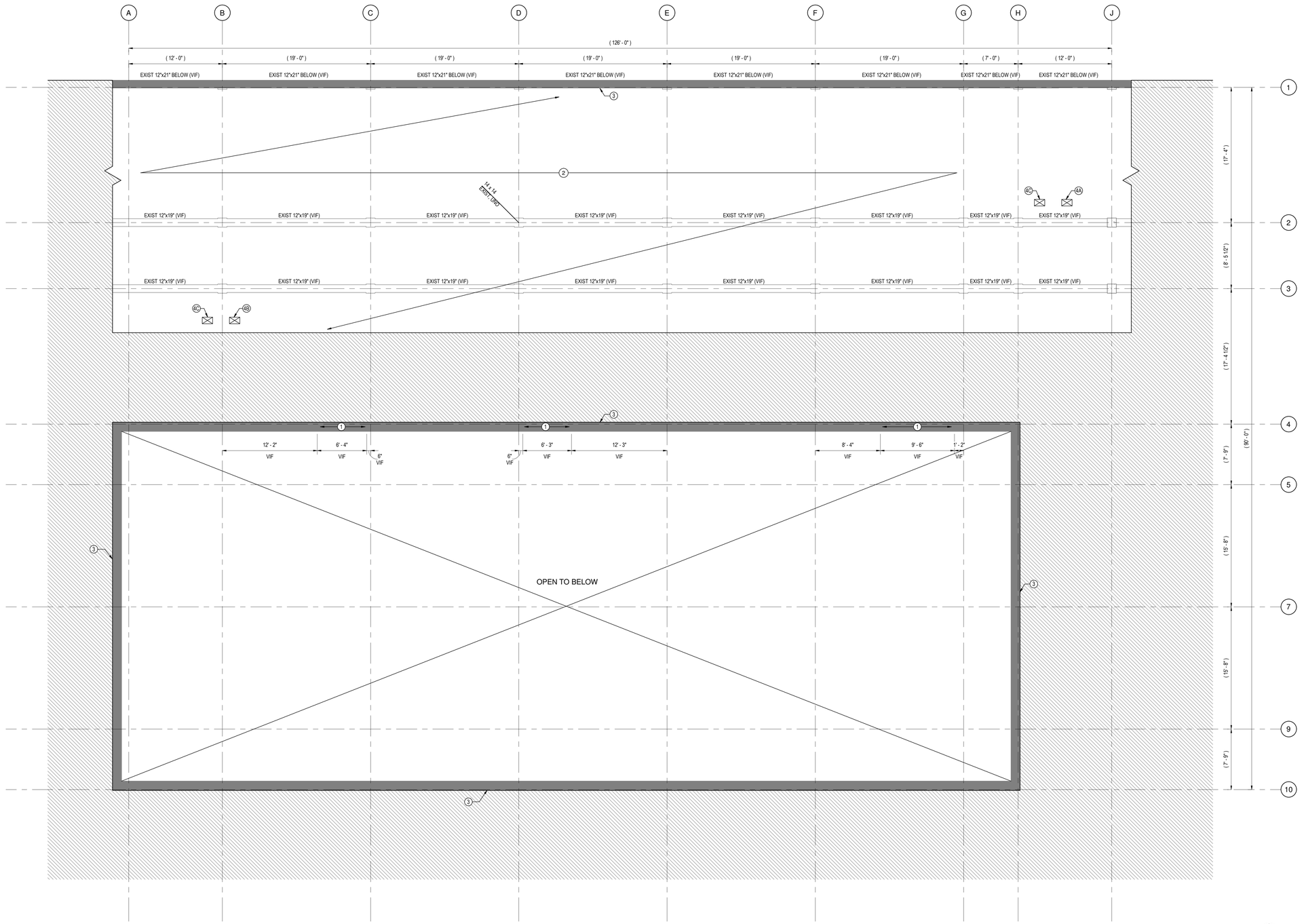
Drawing Number
SD101
Drawing # 24 OF 190

Veterans Health Administration

VA U.S. Department of Veterans Affairs

12/19/2022 3:42:22 PM

VA FORM 08-6231



1 NEW FIRST FLOOR/PARTIAL EXISTING FRAMING DEMO PLAN

3/16" = 1'-0"

GENERAL DEMOLITION SHEET NOTES

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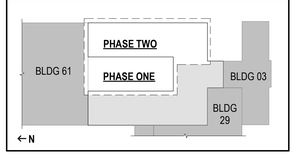
K. (X'-X") INDICATES EXIST DIMENSIONS OR ELEVATIONS. GENERAL CONTRACTOR TO FIELD VERIFY EXIST CONDITIONS, ELEVATIONS AND DIMENSIONS.

(#) GROUND FLOOR DEMO PLAN NOTES:

1. NEW/EXTENDED OPENING IN EXISTING WALL. COORD SIZE AND LOCATION WITH ARCH AND EXISTING CONDITIONS. RE: SF301 FOR ELEVATIONS.
2. EXIST 6" REINF. CONCRETE SLAB TO REMAIN.
3. EXISTING CONCRETE SPANDRELS WITH CMU INFILL WALLS, VIF.
4. NEW OPENING IN EXISTING CONCRETE SLAB. COORDINATE EXACT SIZE AND LOCATION WITH ARCH AND MEP. OPENINGS OCCUR AT MULTIPLE LEVELS AS FOLLOWS:
   - 4A. THROUGH 1st & 2nd FLOOR SLAB.
   - 4B. THROUGH 2nd FLOOR SLAB ONLY.
   - 4C. THROUGH ROOF SLAB ONLY.

KEY PLAN - GROUND FLOOR & FIRST FLOOR



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No.	REVISION DESCRIPTION	DATE

12/19/2022 3:42:23 PM

VA FORM 08-6231

CONSULTANTS:

STRUCTURAL / CIVIL ENGINEER	MECH / ELEC / PLUMB / TECH ENGR	FIRE PROTECTION ENGINEER
H2B, INC. (FIRM REG #: E-3405) 1225 N. LOOP WEST, SUITE 800 HOUSTON, TX 77008 (713) 864-2900	SPUR DESIGN 25219 MADISON AVENUE, SUITE 100 KANSAS CITY, MO 64108 (913) 369-7200	POOLE FIRE PROTECTION, INC. 19910 WEST 161ST STREET OLATHE, KANSAS 66062 (913) 829-8690
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ARCHITECT:

spur DESIGN

SPUR DESIGN, LLC
312 SW 25TH STREET
Oklahoma City, OK 73109
(405) 842-6100

KS ARCH REG. NO. A-930, EXP. 12/31/2021
KS ENGR REG. NO. E-2586, EXP. 12/31/2021

STAMP:

Drawing Title: **FIRST FLOOR DEMO PLAN**

VA Health Care System Approval:

Project Title: **CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS**

Location: 5500 EAST KELLOGG AVENUE, WICHITA, KANSAS 67218

Date: **12/21/2022** | Checked: SJB | Drawn: ZAF

Project Number: **589-704**

Building Number: **26**

Drawing Number: **SD102**

Drawing # 25 OF 190

Veterans Health Administration

VA | U.S. Department of Veterans Affairs

GENERAL SHEET NOTES

A. REFERENCE SHEET SA001 FOR STRUCTURAL GENERAL NOTES AND SA030 AND SA050 FOR TYPICAL STRUCTURAL DETAILS. REVIEW NOTES & DETAILS FOR APPLICABILITY.

B. SEE ARCHITECTURAL DRAWING FOR DETAILS & DIMENSIONS NOT SHOWN.

C. FINISH FLOOR (FF) ELEVATION = 0'-0" UNO (COORDINATE FF ELEVATION W/ EXIST ADJACENT B26 FF).

D. FOOTINGS DENOTED ON PLAN BY "FX"; REFER TO SCHEDULE ON THIS SHEET FOR SIZE AND REINFORCEMENT. REFER TO PLAN FOR ELEVATION.

E. (X'-X") INDICATES EXIST DIMENSIONS OR ELEVATIONS. GENERAL CONTRACTOR TO FIELD VERIFY EXISTING CONDITIONS, ELEVATIONS AND DIMENSIONS.

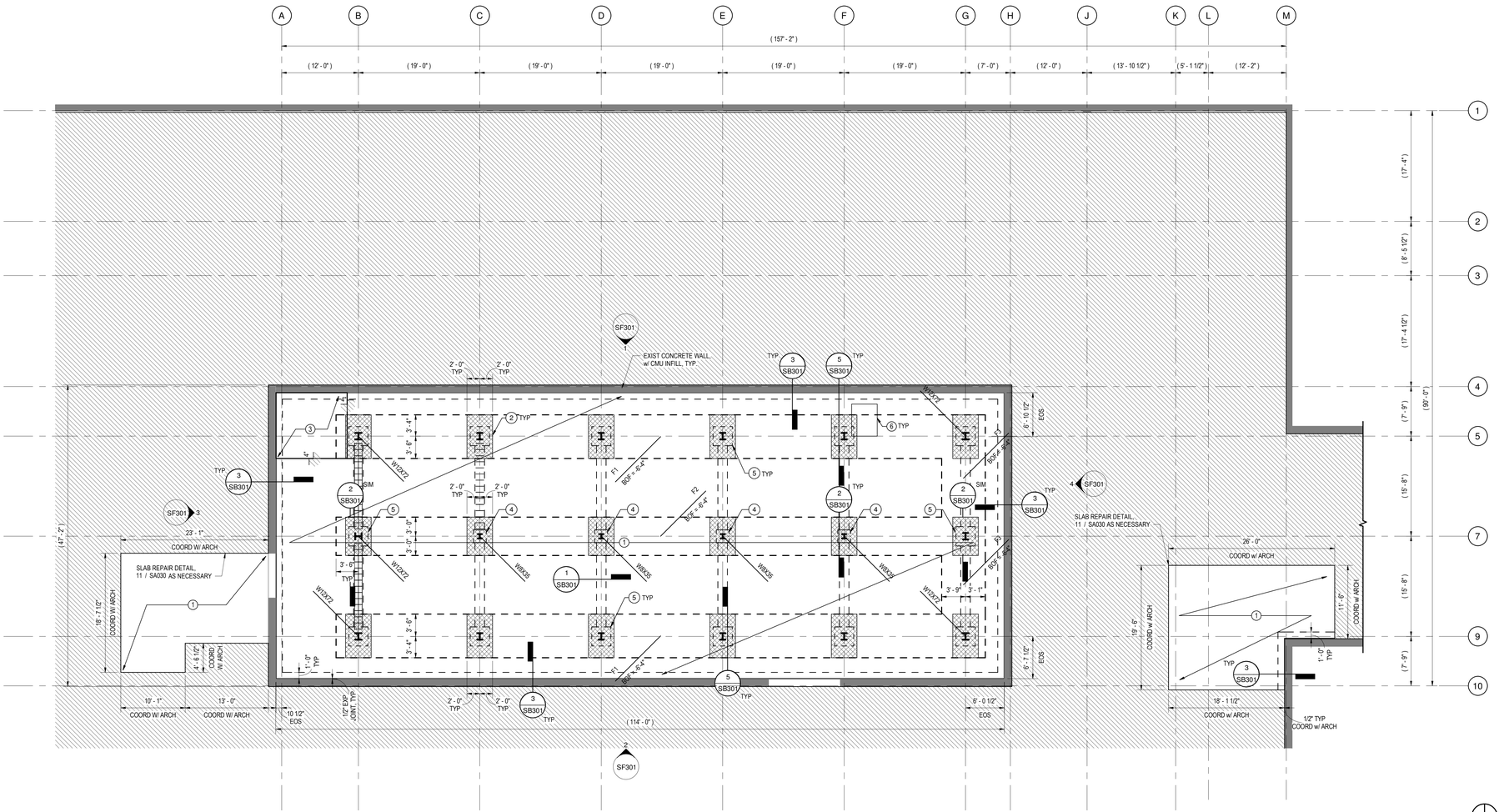
F. ALL COLUMNS ARE W12X58, UNO.

(#) **FOUNDATION PLAN NOTES:**

1. NEW 7" THICK CONCRETE SLAB ON GRADE W/ #4 @ 16" OCEW ON 3" HIGH CHAIR @ 42" OCEW. PLACE SLAB ON 15 MIL WATER VAPOR BARRIER OVER COMPACTED SELECT FILL - SEE SOIL REPORT. SEE STRUCTURAL GENERAL NOTES AND TYPICAL DETAILS FOR JOINT REQUIREMENTS.
2. REINFORCEMENT WITHIN THIS REGION SHALL BE #6 @ 5" OC, EW, T&B.
3. 4" SLAB DEPRESSION. SEE DETAIL 10/SAC030. VERIFY w/ ARCH FOR LOCATION AND SIZE.
4. 2'-0" x 2'-0" CONCRETE PEDESTAL, TYPICAL @ 4 PLACES.
5. 3'-0" x 3'-0" CONCRETE PEDESTAL, TYPICAL @ 14 PLACES.
6. MECH PAD, RE: ARCH FOR SIZE AND LOCATION. RE: 6 / SB301

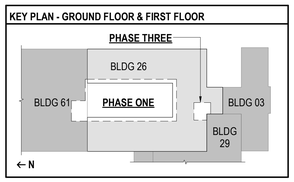
SCHEDULE - SPREAD FOOTING

Type Mark	WIDTH	THICK	REINF
F1	6' - 10"	1' - 3"	#6 @ 10" OC, EW, T&B, UNO
F2	6' - 0"	1' - 3"	#6 @ 10" OC, EW, T&B, UNO
F3	6' - 10"	1' - 3"	#6 @ 10" OC, EW, T&B, UNO



1 GROUND FLOOR/FOUNDATION PLAN
1/8" = 1'-0"

KEY PLAN - GROUND FLOOR & FIRST FLOOR



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SARAH J. BRIDGES LICENSED 28649 KANSAS PROFESSIONAL ENGINEER 12/21/22

Drawing Title
GROUND FLOOR/FOUNDATION PLAN

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
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Date **12/21/2022** | Checked SJB | Drawn ZAF

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Building Number **26**

Drawing Number **SB101**

Drawing # 26 OF 190

Veterans Health Administration

U.S. Department of Veterans Affairs

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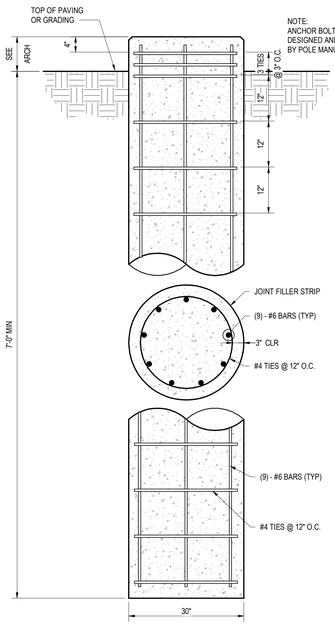
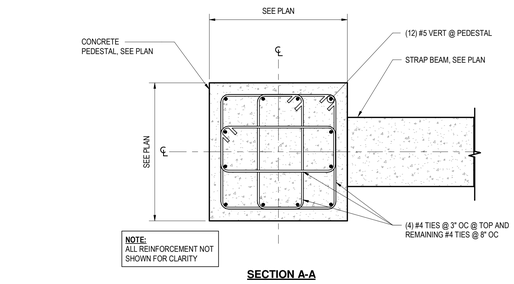


NOTES:

1. LIGHT POLE FOUNDATION DESIGNED FOR 20 FOOT TALL POLE CAPABALE OF RESISTING PRESSURES INDUCED BY 3 SECOND GUST AT 123 MPH DESIGN WIND SPEED.
2. CONCRETE SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF 3500 PSI WITH A MAXIMUM AGGREGATE SIZE OF 3/4" AND A MAXIMUM WATER/CEMENT RATIO OF 0.54.
3. ALL REINFORCEMENT TO HAVE 60 KSI YIELD STRENGTH.
4. JOINT FILLER STRIPS FOR JOINTS SHALL CONFORM TO ASTM D-1751 OR D-1752. JOINT FILLER SHALL BE 1/2" THICK MINIMUM UNLESS SHOWN OTHERWISE ON DRAWINGS.
5. DIMENSIONS FOR BOLT CIRCLE SPACING SHALL MEET MANUFACTURER SPECIFICATIONS.
6. HOLES FOR THE LIGHT POLE FOUNDATION TO BE DRILLED USING AN AUGER OF THE APPROPRIATE DIMENSION. OTHER EXCAVATION METHODS WILL COMPROMISE THE STRUCTURAL INTEGRITY OF THE SOIL AND SHOULD NOT BE USED.

8 LIGHT POLE DETAIL
NTS



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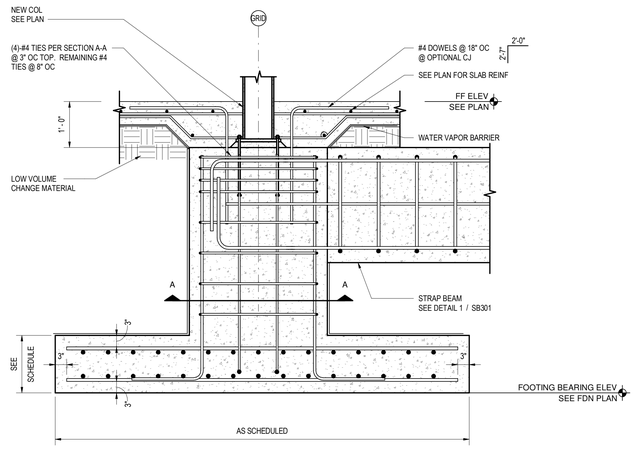
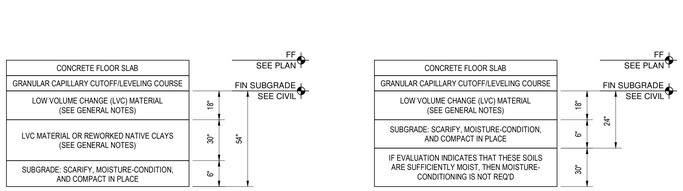
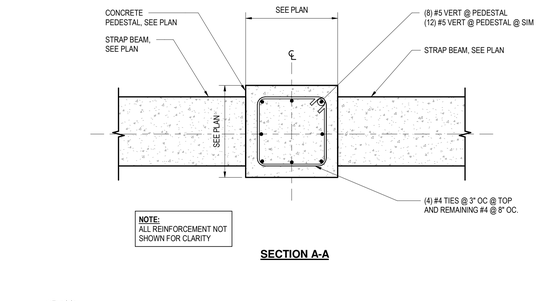


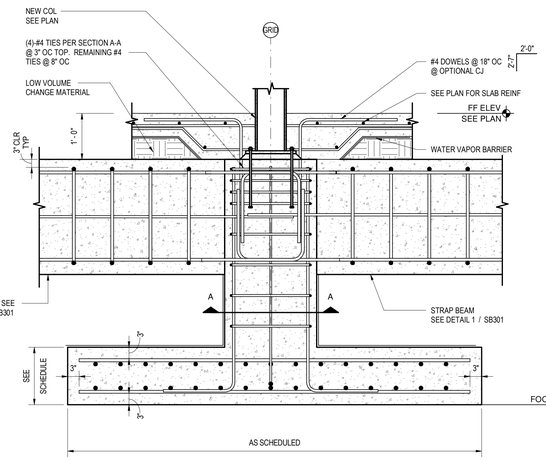
5 TYPICAL PEDESTAL DETAIL AT EXTERIOR COLUMN
NTS



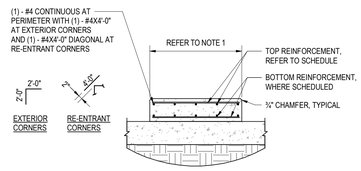
4 SUBGRADE PREP DIAGRAM
NTS



SECTION A-A



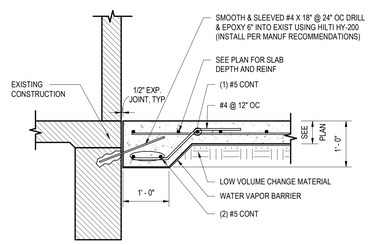
2 TYPICAL PEDESTAL DETAIL AT INTERIOR COLUMN
NTS



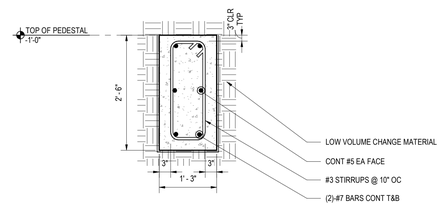
HOUSEKEEPING PAD REINFORCEMENT SCHEDULE		
PAD THICKNESS	TOP REINFORCEMENT	BOTTOM REINFORCEMENT
T-4"	WWR 4x4-W2.9xW2.9 OR #3@12" EACH WAY	NONE
4"< T< = 6"	WWR 4x4-W4xW4 OR #4@18" EACH WAY	NONE
6" < T < = 8"	WWR 4x4-W6xW6 OR #4@12" EACH WAY	NONE
8" < T < = 12"	#4@18" EACH WAY	#4@18" EACH WAY
12" < T < = 16"	#4@12" EACH WAY	#4@12" EACH WAY

NOTE:
1. REFER TO ARCHITECTURAL, STRUCTURAL, OR MEP DRAWING FOR HOUSEKEEPING PAD PLAN DIMENSIONS AND THICKNESS (4" MINIMUM THICKNESS).
2. CONTRACTOR SHALL COORDINATE DIMENSIONS AND OTHER SPECIAL REQUIREMENTS WITH EQUIPMENT MANUFACTURERS AND PROVIDE WHERE REQUIRED WHETHER SHOWN ON STRUCTURAL DRAWING OR NOT.
3. PROVIDE ADDITIONAL LAYER OF REINFORCING FOR PAD THICKNESS GREATER THAN 18".

6 TYPICAL HOUSEKEEPING PAD OVER SOG
NTS



3 TYPICAL TURNDOWN AT EXISTING
NTS



1 TYPICAL STRAP BEAM DETAIL
NTS

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POOLE FIRE PROTECTION, INC.
19910 WEST 161ST STREET
OLATHE, KANSAS 66062
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RIVERFRONT HEALTH & SAFETY
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HEALTHCARE PLANNER
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3210 GULF BLVD, UNIT 304
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ARCHITECT:

spur DESIGN

SPUR DESIGN, LLC
312 SW 25TH STREET
Oklahoma City, OK 73109
(405) 842-6100

KS ARCH REG. NO. A-930, EXP. 12/31/2021
KS ENGR REG. NO. E-2586, EXP. 12/31/2021

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SARAH J. BRIGGS
LICENSED
28649
KANSAS
PROFESSIONAL ENGINEER
12/21/22

Drawing Title
FOUNDATION DETAILS

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date: **12/21/2022** | Checked: SJB | Drawn: ZAF

Project Number: **589-704**

Building Number: **26**

Drawing Number: **SB301**

Drawing # 27 OF 190

Veterans Health Administration

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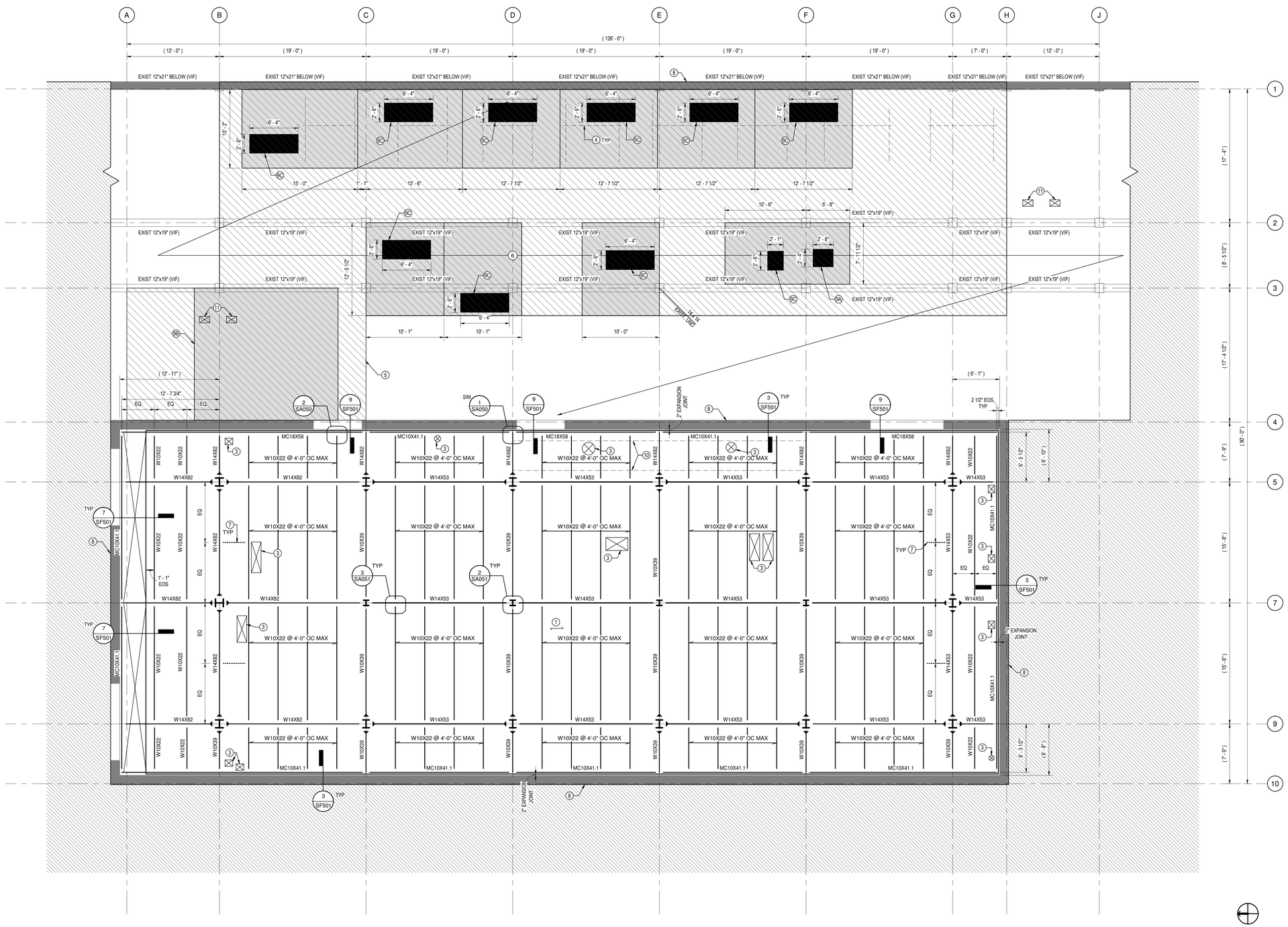


1 NEW FIRST FLOOR/PARTIAL EXISTING FRAMING PLAN
3/16" = 1'-0"

GENERAL SHEET NOTES

A. REFERENCE SHEET SA001 FOR STRUCTURAL GENERAL NOTES AND SA050 FOR TYPICAL STRUCTURAL DETAILS. REVIEW NOTES & DETAILS FOR APPLICABILITY.

B. SEE ARCHITECTURAL DRAWING FOR DETAILS & DIMENSIONS NOT SHOWN.

C. TOC ELEV = 10' - 8"
TOS ELEV = 10'- 2 1/2"
TOS = BOTTOM OF DECK

D. (X'-X") INDICATES EXIST DIMENSIONS OR ELEVATIONS. GENERAL CONTRACTOR TO FIELD VERIFY EXIST CONDITIONS,ELEVATIONS AND DIMENSIONS.

E. EXISTING CONCRETE BEAMS AND REINFORCING MAY NOT BE CUT FOR INSTALLATION OF NEW MEP SYSTEMS OR IN ANY OTHER INSTANCE. CONTRACTOR TO NOTIFY EOR IF A CONFLICT EXISTS.

(#) **2ND FLOOR AND LOW ROOF PLAN NOTES:**

1. 5 ½" NW NON-COMPOSITE SLAB (4" CONCRETE TOPPING OVER 1 ½" 20 GA NON-COMPOSITE DECK), REINF W/ WWF 4x4-W2.9xW2.9.
2. SEE DETAIL 1 / SF501 TYP AT ALL SLAB EDGES, UNO.
3. TYPICAL FLOOR OPENING, COORDINATE LOCATION W/ MECHANICAL AND ARCH. FOR FLOOR OPENING DETAIL, RE: 4 / SA051 AND 5 / SA051
4. INDICATES PATIENT LIFT (8) LOCATIONS. COORD LOCATION WITH EXISTING CONDITIONS, ARCH AND MEP. ASSUMED WEIGHT = 1,000 LBS.
5. FIBER REINF POLYMER TO SUPPORT ADDITIONAL LOADING AS NOTED ON PLAN, DESIGNED BY OTHERS.
6. EXIST 6" REINF. CONCRETE SLAB.
7. BEAM BOTTOM FLANGE BRACE, RE:6 / SF501
8. EXISTING CONCRETE SPANDRELS WITH CMU INFILL WALLS, VIF.
9. NEW EQUIPMENT LOADING AREA. SEE PLAN FOR DIMENSIONS. EQUIPMENT CAN BE ARRANGED TO ANY LOCATION WITHIN GREY REGION.
   9A. 300 LBS
   9B. 85 PSF
   9C. 800 LBS
10. INDICATES KITCHEN HOOD SUPPORT BELOW RE: 7 / SA051. COORDINATE EXACT LOCATION WITH ARCH AND MEP.
11. FIBER REINF POLYMER TO SUPPORT OPENING, RE: 8 / SA051. REFER TO SD102 FOR OPENING LOCATIONS.

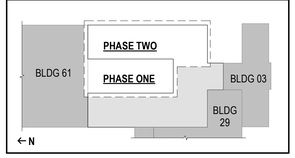
BEAM SIZE → W8x10
INDICATES MOMENT CONNECTION

BEAM LEGEND

SEE DETAIL 4 / SA050 FOR FLANGE MOMENT CONNECTION DETAIL, UNO.

SEE DETAIL 5 / SA050 FOR WEB MOMENT CONNECTION DETAIL, UNO.

KEY PLAN - GROUND FLOOR & FIRST FLOOR



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No.	REVISION DESCRIPTION	DATE

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CONSULTANTS:

STRUCTURAL / CIVIL ENGINEER
H2B, INC. (FIRM REG #: E-3405)
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HOUSTON, TX 77008
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SHAWN J. BRISCOE LICENSED 28649 KANSAS PROFESSIONAL ENGINEER 12/21/22

Drawing Title
FIRST FLOOR FRAMING PLAN

VA Health Care System Approval:

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CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
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Date **12/21/2022** | Checked SJB | Drawn ZAF

Project Number **589-704**

Building Number **26**

Drawing Number **SF102**

Drawing # 28 OF 190

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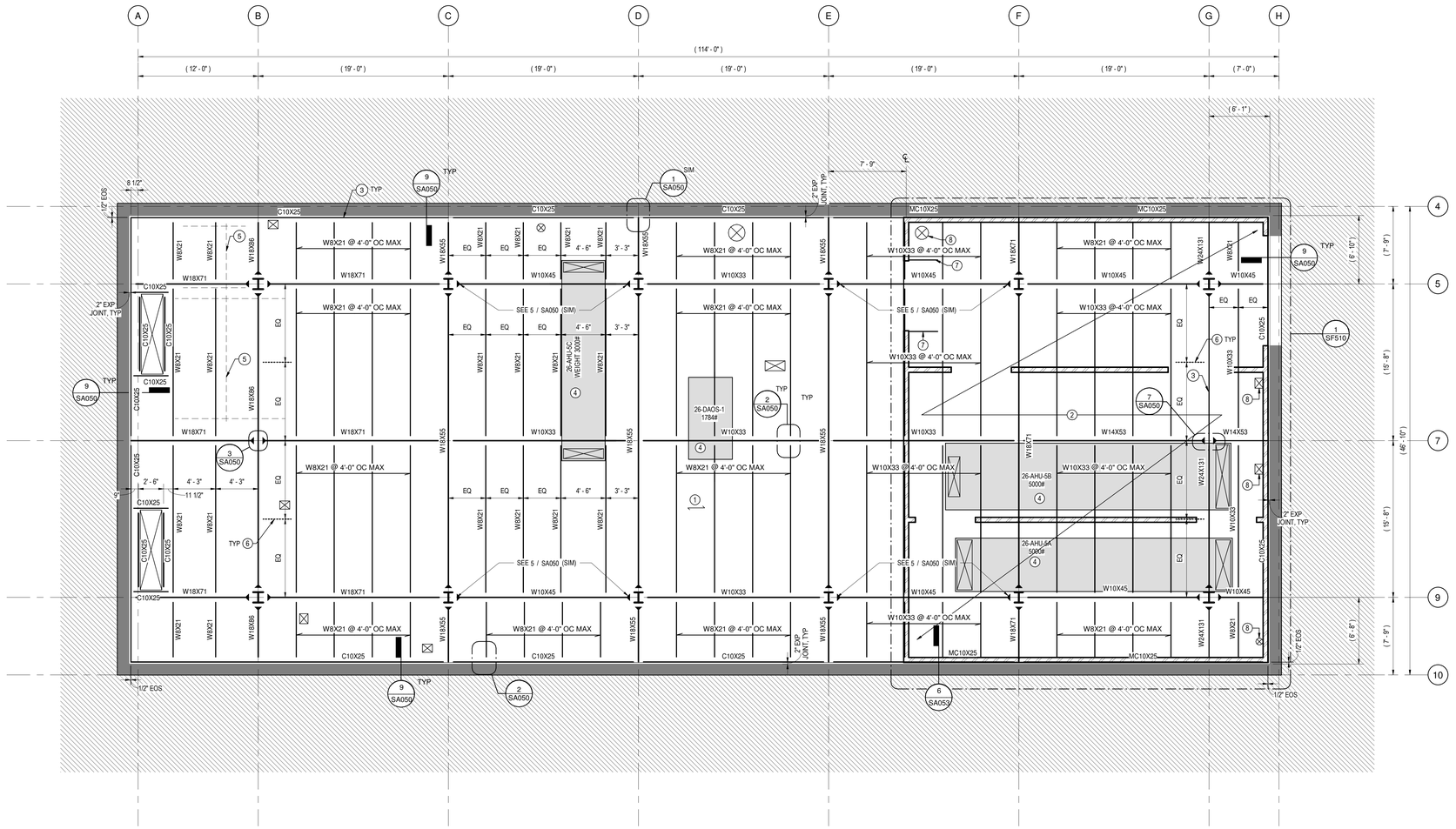


1 ROOF FRAMING PLAN

3/16" = 1'-0"

GENERAL SHEET NOTES

A. REFERENCE SHEET SA001 FOR STRUCTURAL GENERAL NOTES AND SA050 FOR TYPICAL STRUCTURAL DETAILS. REVIEW NOTES & DETAILS FOR APPLICABILITY.

B. SEE ARCHITECTURAL DRAWING FOR DETAILS & DIMENSIONS NOT SHOWN.

C. ALL ROOF OPENINGS SHALL BE FRAMED WITH ANGLE FRAMES PER THE TYPICAL DETAILS.

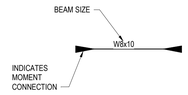
D. (X'-X") INDICATES EXIST DIMENSIONS OR ELEVATIONS. GENERAL CONTRACTOR TO FIELD VERIFY EXIST CONDITIONS, ELEVATIONS, AND DIMENSIONS.

E. PROVIDE L4X4X1/4 CONT AT PERIMETER OF ROOF. SEE 9 / SA050.

F. TOS = 22'-5"
TOS = BOTTOM OF DECK

(#) **ROOF PLAN NOTES:**

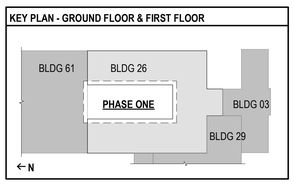
1. 3" 18 GA TYPE N METAL DECK. REFER TO GENERAL NOTES FOR FASTENING REQUIREMENTS.
2. 3 ½" NW NON-COMPOSITE SLAB (3" TOPPING OVER 0.6C 20 GA DECK. REINF W/ 4x4-W2.9xW2.9.
3. SEE DETAIL 6 / SA050 FOR OPENING IN BEAM WEB. COORDINATE LOCATION WITH PLUMBING.
4. INDICATES RTU. COORD LOCATION WITH ARCH AND MEP. SEE DETAIL 8 / SA050 FOR OPENING IN DECK.
5. INDICATES PATIENT LIFT (2) LOCATIONS. SEE DETAIL 4 / SF501. COORD LOCATION WITH EXISTING CONDITIONS, ARCH AND MEP. ASSUMED WEIGHT = 1,000 LBS.
6. BEAM BOTTOM FLANGE BRACE, RE: 6 / SF501
7. W8x18, LOCATE UNDER STAIR STRINGER, RE: ARCH AND 5 / SF501
8. TYPICAL FLOOR OPENING, COORDINATE LOCATION W/ MECHANICAL AND ARCH. FOR FLOOR OPENING DETAIL, RE: 4 / SA051 AND 5 / SA051



BEAM LEGEND

SEE DETAIL 4 / SA050 FOR FLANGE MOMENT CONNECTION DETAIL, UNO.

SEE DETAIL 5 / SA050 FOR WEB MOMENT CONNECTION DETAIL, UNO.



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STAMP:

SHAN J. BRISCOE LICENSED 28649 KANSAS PROFESSIONAL ENGINEER
12/21/22

Drawing Title
ROOF FRAMING PLAN

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date	Checked	Drawn
12/21/2022	SJB	ZAF

Project Number
589-704

Building Number
26

Drawing Number
SF103
Drawing # 29 OF 190

Veterans Health Administration

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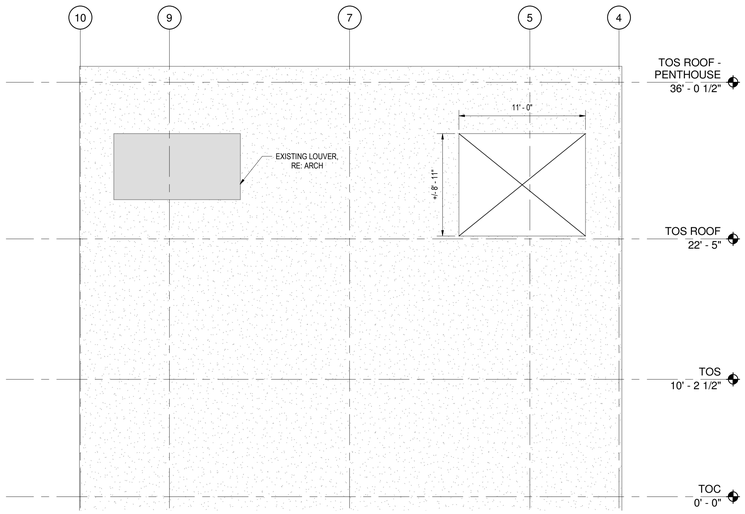
GENERAL SHEET NOTES:

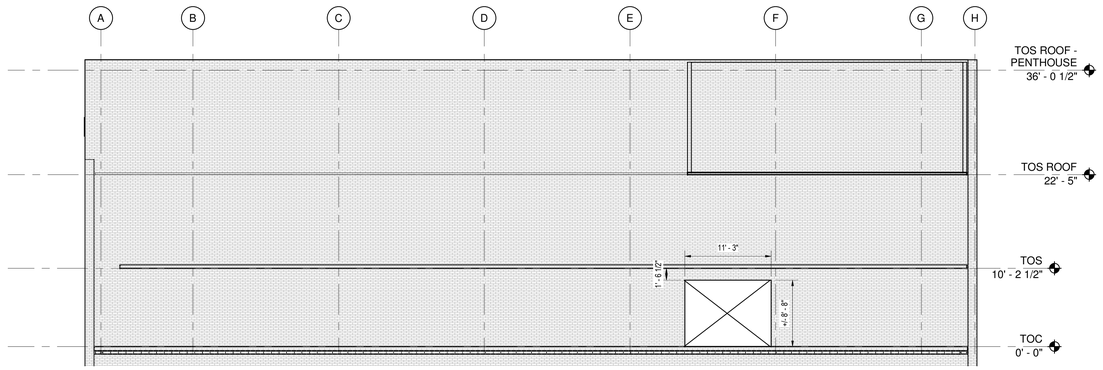
A. GC TO COORDINATE LOCATIONS OF NEW OPENINGS IN EXISTING CMU WALL WITH EXISTING CONDITIONS AND ARCH.

B. REMOVE EXISTING CMU WALL INFILL AND BRICK CLADDING UP TO THE BOTTOM OF EXISTING CONCRETE BEAM. CONCRETE BEAM AND REINFORCING SHALL NOT BE CUT. PROVIDE CFMF STUD INFILL AS REQUIRED PER OPENING HEIGHT REQUIREMENT.

C. REMAINING BRICK CLADDING ABOVE OPENINGS SHALL BE SUPPORTED BY NEW ANGLE ATTACHED TO EXISTING CONCRETE BEAM. SEE DETAIL 5 THIS SHEET.



4 ELEVATION - SOUTH WALL
NTS

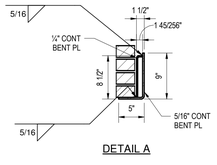


2 ELEVATION - WEST WALL
1/8" = 1'-0"

LOOSE LINTEL SCHEDULE			
MARK	OPENING SIZE	LINTEL SIZE	BRG LEN
L1	UP TO 1'-8"	¼" PL X WALL WIDTH - ½"	4"
L2	1'-9" TO 3'-0"	L 3-½" x 3-½" x ¼"	4"
L3	3'-1" TO 4'-5"	L 4" x 3-½" x 5/16"	4"
L4	4'-6" TO 6'-3"	L 5" x 3-½" x 3/8"	4"
L5	6'-4" TO 8'-0"	L 6" x 3-½" x 3/8"	8"
L6	8'-1" TO 12'-0"	5/16" & 1/4" BENT PL (SEE DTL A)	8"

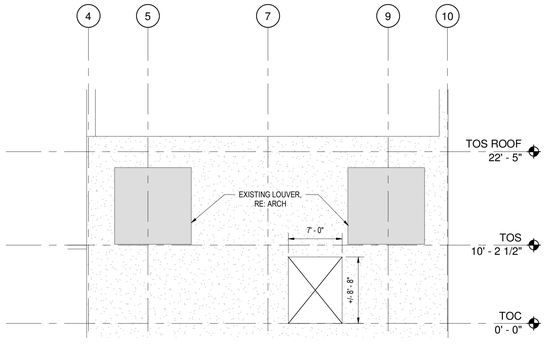
NOTES:

1. HEIGHT OF MASONRY ABOVE LINTEL MUST BE MINIMUM OF ⅓ THE OPENING WIDTH.
2. PROVIDE ONE STEEL LINTEL FOR EACH 4" THICKNESS OF MASONRY. (FOR EXAMPLE: 3 ANGLES FOR 12" OF WALL).
3. LINTELS SPECIFICALLY CALLED OUT ON DRAWINGS SHALL GOVERN OVER THIS SCHEDULE.
4. ALL STEEL ANGLES SHALL BE GALVANIZED AND SHALL HAVE SHORT LEG HORIZONTAL (SLH).
5. L6 REQUIRES A 2" MIN AIR GAP TO FIT IN WALL CAVITY.

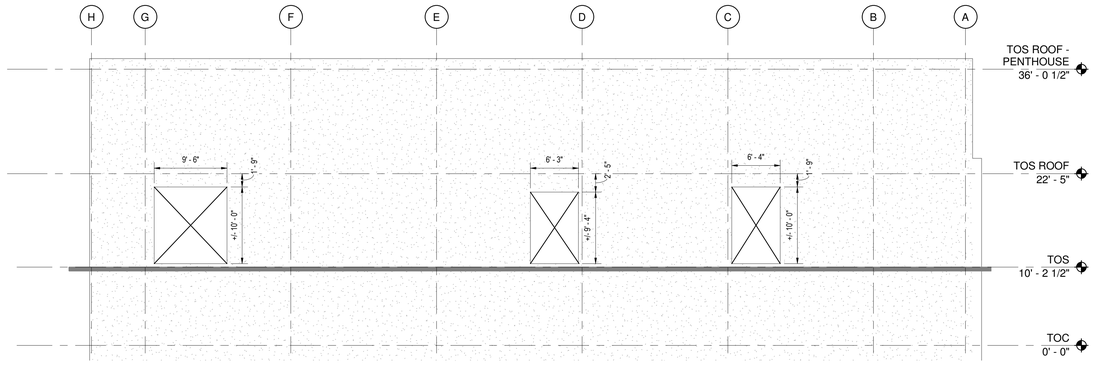


DETAIL A

5 BRICK LINTEL SCHEDULE
NTS



3 ELEVATION - NORTH WALL
1/8" = 1'-0"



1 ELEVATION - EAST WALL
1/8" = 1'-0"

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CONSULTANTS:

STRUCTURAL / CIVIL ENGINEER	MECH / ELEC / PLUMB / TECH ENGR	FIRE PROTECTION ENGINEER
H2B, INC. (FIRM REG #: E-3405) 1225 N. LOOP WEST, SUITE 800 HOUSTON, TX 77008 (713) 864-2900	SPUR DESIGN 25219 MADISON AVENUE, SUITE 100 KANSAS CITY, MO 64108 (913) 369-7200	POOLE FIRE PROTECTION, INC. 19910 WEST 161ST STREET OLATHE, KANSAS 66062 (913) 829-8690
INDUSTRIAL HYGENIST	HEALTHCARE PLANNER	PHYSCIAL SECURITY
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ARCHITECT:

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SHAWN J. BRIGGS LICENSED 28649 KANSAS PROFESSIONAL ENGINEER 12/21/22

Drawing Title
EXISTING BUILDING SECTIONS

VA Health Care System Approval:

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CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

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SF301
Drawing # 30 OF 190

Veterans Health Administration

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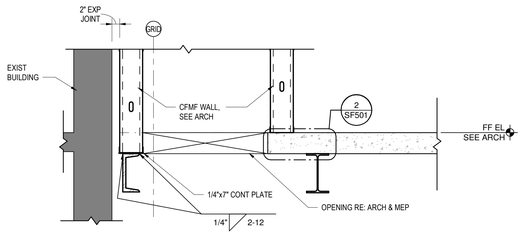
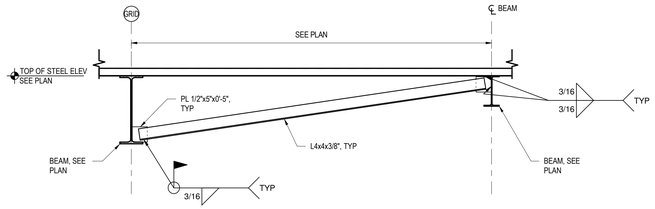
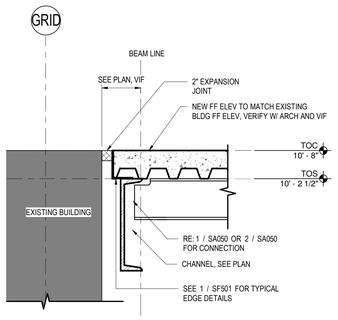


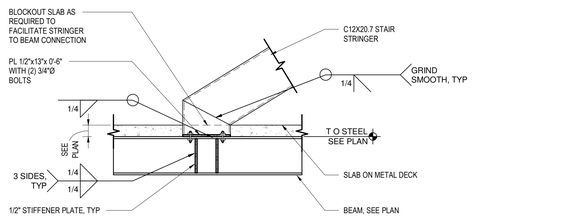
7 OPENING IN SLAB CONDITION
NTS



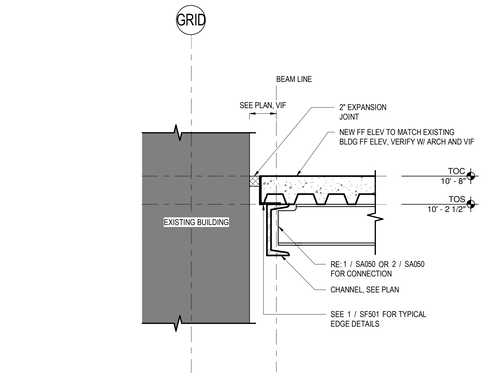
6 TYPICAL BOTTOM FLANGE BRACE
NTS



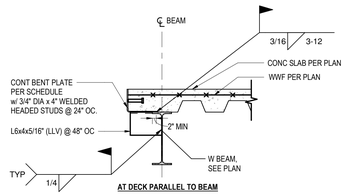
9 TYPICAL SLAB DETAIL AT EXISTING OPENING
NTS



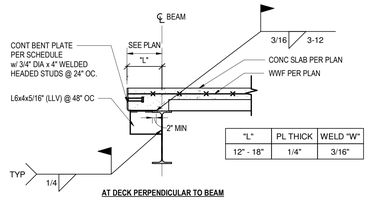
5 STAIR STRINGER CONNECTION
NTS



3 TYPICAL SLAB DETAIL AT EXISTING BUILDING
NTS



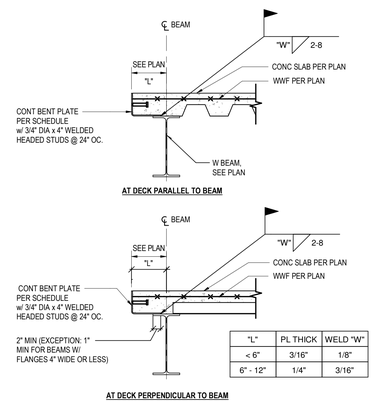
AT DECK PARALLEL TO BEAM



"L"	PL THICK	WELD "W"
12" - 18"	1/4"	3/16"

AT DECK PERPENDICULAR TO BEAM

2 TYPICAL SLAB EDGE
NTS

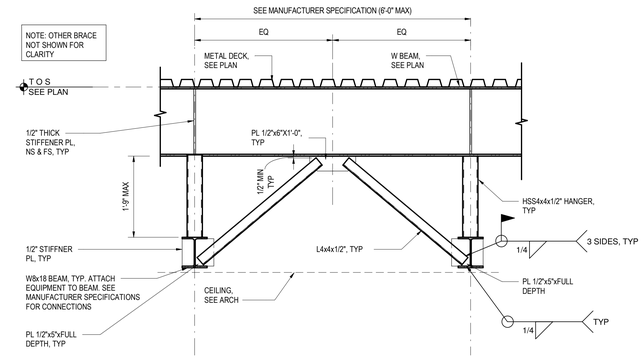


AT DECK PARALLEL TO BEAM

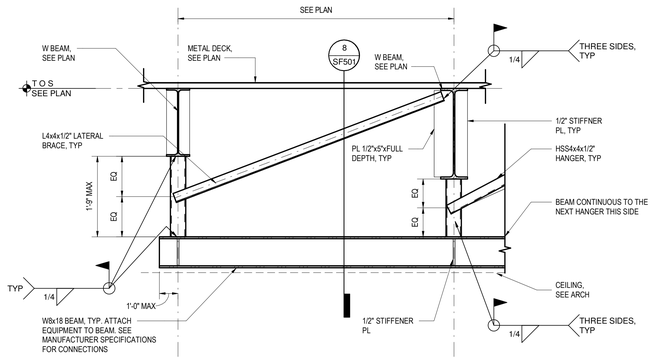
"L"	PL THICK	WELD "W"
< 6"	3/16"	1/8"
6" - 12"	1/4"	3/16"

AT DECK PERPENDICULAR TO BEAM

1 TYPICAL SLAB EDGE
NTS



8 EQUIPMENT HANGER DETAIL
NTS



4 EQUIPMENT HANGER DETAIL
NTS

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Drawing # 31 OF 190

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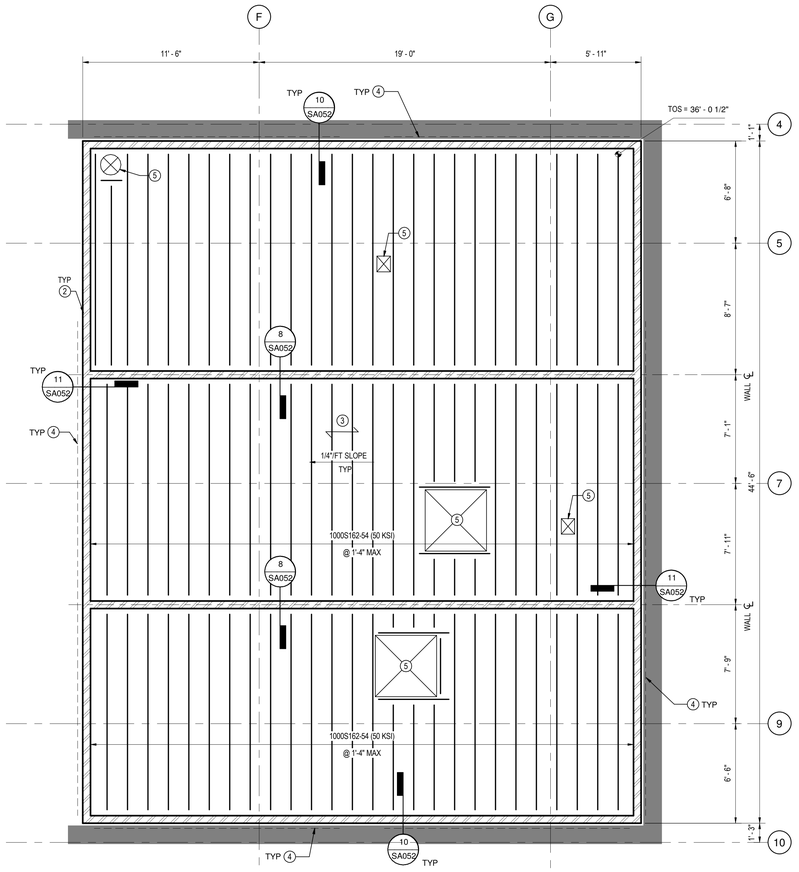
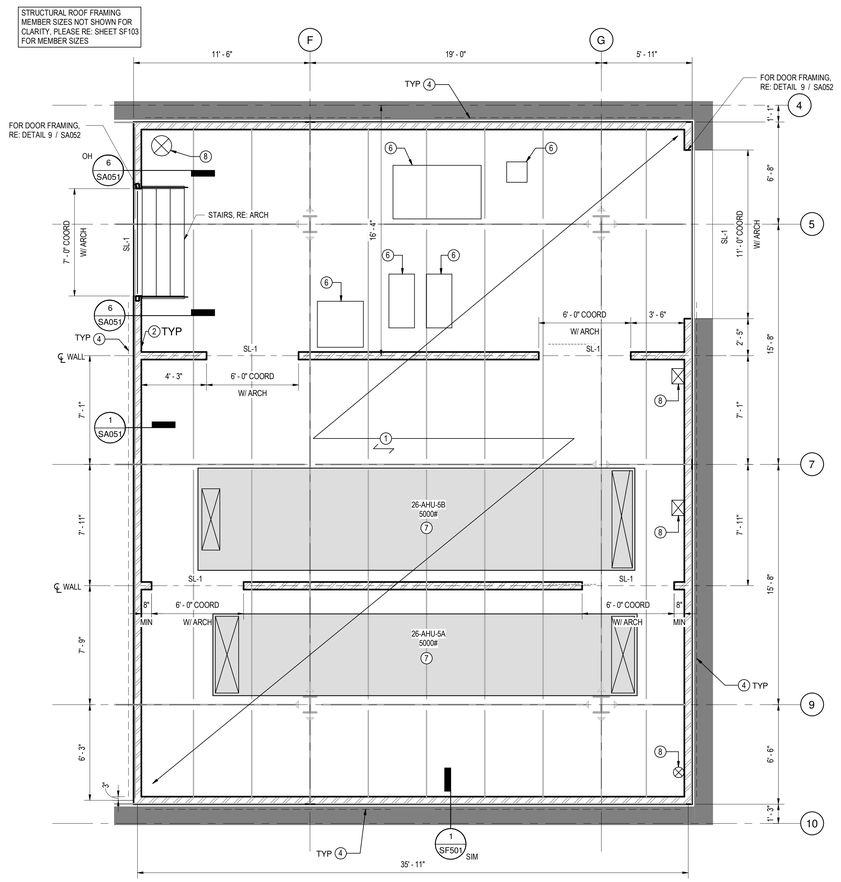


2 MECHANICAL PENTHOUSE ROOF FRAMING PLAN

1/4" = 1'-0"



1 MECHANICAL PENTHOUSE PLAN

1/4" = 1'-0"

GENERAL SHEET NOTES

A. REFERENCE SHEET SA001 FOR STRUCTURAL GENERAL NOTES AND SA051 FOR TYPICAL STRUCTURAL DETAILS. REVIEW NOTES & DETAILS FOR APPLICABILITY.

B. SEE ARCHITECTURAL DRAWING FOR DETAILS & DIMENSIONS NOT SHOWN.

C. ALL ROOF OPENINGS SHALL BE FRAMED WITH ANGLE FRAMES PER THE TYPICAL DETAILS.

D. (X'-X") INDICATES EXIST DIMENSIONS OR ELEVATIONS. GENERAL CONTRACTOR TO FIELD VERIFY EXIST CONDITIONS, ELEVATIONS AND DIMENSIONS.

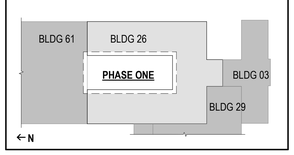
E. MECHANICAL PENTHOUSE STRUCTURE SHALL BE DESIGNED TO LIMIT ITS LATERAL DEFLECTION TO 1" MAX.

F. TOS = BOTTOM OF DECK.

(#) MECHANICAL PENTHOUSE PLAN NOTES:

1. 3 ½" N/W NON-COMPOSITE SLAB (3" TOPPING OVER 0.6C 20 GA DECK. REINF w/ 4x4-W2.9xW2.9).
2. NEW CFMF STUD WALL w/ 600S162-54 (Fy=50 KSI) @ 16" OC, BY OTHERS.RE: DETAIL 9 / SA052.
3. 1.5" GA 20 TYPE B METAL DECK. REFER TO GENERAL NOTES FOR FASTENING REQUIREMENTS.
4. STRAP BRACE AT (8) PLACES, RE: 1 / SA053
5. CFMF FRAMED ROOF OPENING. RE: 7 / SA053. PROVIDE HEADER ONLY FOR OPENINGS LESS THAN 10" WIDE. SEE MECHANICAL FOR LOCATIONS.
6. 4" THICK HOUSEKEEPING PAD, COORDINATE EXACT LOCATION AND SIZE WITH MEP AND ARCH, RE: 9 / SA051
7. INDICATES RTU. COORD LOCATION WITH ARCH AND MEP. SEE DETAIL 8 / SA050 FOR OPENING IN DECK.
8. TYPICAL FLOOR OPENING, COORDINATE LOCATION W/ MECHANICAL AND ARCH. FOR FLOOR OPENING DETAIL, RE: 4 / SA051 AND 5 / SA051

KEY PLAN - GROUND FLOOR & FIRST FLOOR



FULLY SPRINKLERED

100% BID SET

No.	REVISION DESCRIPTION	DATE

12/19/2022 3:42:31 PM

VA FORM 08-6231

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28649
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PROFESSIONAL ENGINEER
12/21/22

Drawing Title
MECHANICAL PENTHOUSE PLANS

VA Health Care System Approval:

Project Title
CONSTRUCT INFILL OF BUILDING 26 AND RENOVATE SPECIALTY CARE CLINICS

Location
5500 EAST KELLOGG AVENUE
WICHITA, KANSAS 67218

Date **12/21/2022** | Checked SJB | Drawn ZAF

Project Number **589-704**

Building Number **26**

Drawing Number **SF510**

Drawing # 32 OF 190

Veterans Health Administration

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